Appendix 7-E SLOPE STABILITY ANALYSIS SEDIMENTATION POND "A"

SLOPE STABILITY ANALYSIS ON EMBANKMENT OF SEDIMENTATION POND "A" BEAR CREEK CANYON MINE SITE CO-OP MINING COMPANY

JULY 1984



Prepared by family for Demander, P. E. Utah P.E. No. 3535

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SECTION 1

In conjunction with the redesign of sedimentation pond "A" of the Bear Creek Canyon Mine site, various in place and laboratory soil tests were performed. Using the parameters obtained from these tests, slope stability analysis were performed to confirm the stability of the 2H:IV interior slope and determine a stable angle for the exterior slope.

SECTION II CONSTRUCTION

On September 29, 1983, I visited the site in conjunction with Wendell Owen of the Co-Op and reviewed in detail the provisions taken at the site during construction. I also reviewed with Mr. Owen the construction procedures which enabled me to determine how the embankment was constructed to its present slope.

SECTION III MATERIAL TESTING

While at the site, using a Troxler 3411B nuclear density gauge, I determined the in-place density of the embankment material. I also obtained moisture density samples and samples of the embankment material, which I returned to the lab for additional testing. The results of these in-place determinations indicate that the average in-place density of the material varied from 89% to 94% of the laboratory obtained T-99 standard proctor.

I submitted a sample of the embankment material to Chen and Associates, a consulting soil and foundation engineering firm, to determine the relationship of the loading to shear stress, and to determine the internal cohesion. These results are included in the Appendix. The material gradation results are also included in the Appendix. The gradation results indicated that the material is free draining confirming the on site tests.

SECTION IV OCCURRENCE OF GROUND WATER

The results of the gradation analysis indicate that the material is free draining. This was further observed at the site through reviewing the existing material in place and performing percolation tests in the embankment. Because of the geology of the mine plan area the embankment is not effected by ground water, but for purposes of analysis the soil was assumed saturated by the water occasionally held in the pond. This assumed extent of saturation provides for conservative results.

SECTION V FACTOR OF SAFETY

A computer model was constructed to analyze the stability of the embankment and the following conditions were assumed.

- 1. The soil below the maximum water elevation on the interior slope to the toe of the exterior slope (soil #2) was assumed saturated.
- 2. In-place soil density of 118 pounds per cubic foot.
- 3 2H:IV angles were used for both interior and exterior slopes.

A computer simulation was then applied to this situation to determine various failure planes. The "Method of Slices" is the basis for the modified Bishop method computer program. Various failure planes were investigated to determine a minimum factor of safety. The results of these computer runs and a copy of the computer listing is attached in the Appendix. The results of these computer simulations indicate that the minimum factor of safety with a .1 g earthquake loading is 3.8 for the interior slope and 2.8 for the exterior slope at the point of maximum length.

SECTION VI CONCLUSIONS

T	1 .	41	1		1		••	1 1 4			• ,•	•
In	conclusion	the	sedimentatio	n n	nana	'' A	٠	embankment	ac	$n\alpha w$	evictin	0.16.
111	conclusion	uic	Scammentatio	пρ	ona	1		Cilibalikilicilt	as	110 00	CAISTII	5 10.

- 1. Free draining.
- 2. No water was encountered in test holes dug in embankments.
- 3. No movement of the embankment has been detected.
- 4. The computer simulation on failure planes indicates that the factor of safety is at least 2.8 with a 0.1 g earthquake loading for 2H:IV slopes for both the interior and exterior of the embankment.

APPENDIX

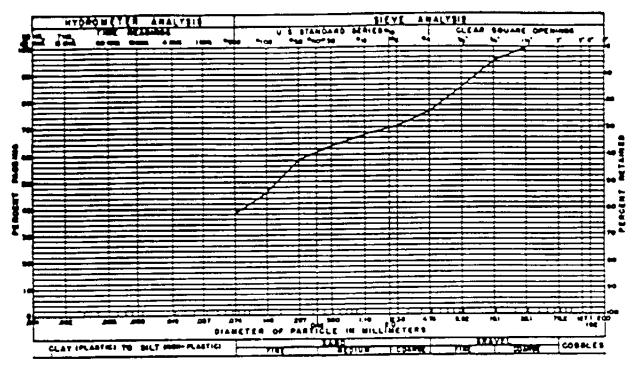
ENGINEERS 10/6/50 Protect Name Come MINE Dat€ Station or Pit Location ct No. _Requisted by ____+W/ #7 ala No 🗕 dikas AS RECEIVED GRADATION Percent Weight Percent Screen SPECS. Size Retained Passing (a) 3 100% \bigcirc Z" \bigcirc "龙" 1132.1 14.0 86.0 3/4" 75.1 10.9 875.3 69 3 1/2 4003 53 3/8 55% **#**4 1112.1 Wet WI. WŁ 4431. 55.6 JI WE 35290 Dry WASHED GRADATION AFTER CRUSHING (2500 GM DRY SAMPLE) Percent Total % Percent Screen Weight SPECS. MOISTURE DETERMINATION Passing Retained Passing Size Retained 91.1 - × 4 #8 8.9 14 .7 19.7 Container & Wet # IO 21.1 2.0 17.5 *e*9.1 Sail Weight (gm.) Container & Dry 695 84.7 17.5 **#16** 4.4 Soil Weight (gm.) H₂O Loss 82.2 **≠** 20 40.7 16.7 2.6 % Moisture 8011 2.0 31.8 **#** 30 16.2 A.A.S.H.O. 479 15.8 77.1 3.0 #40 Classification # 50 71.4 91.0 5.3 15.2 7.00 22.1 49.3 #100 14,0 9.2 9.7 146.6 40.1 Wt. before washing. # 200 40.1 635.7 \bigcirc Wt. after washing .. fotal Wt COPIES TO: 15868

Tested by KIEL I. LEGISLE

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ject No rámpie No .	= 1601	_ \$100 1 CC	-	Station or Pit Requisted by	,		
	•	Ediment		<u> </u>			
	AS REG	CEIVED GR	ADATION				
Screen Size	Weight (a)	Percent Retained	Percent Passing	SPECS.			
3*			, 0331119				
2"							
1 "	0	0	100%				
3/4"	62.0	00	79.2				
1/2"	943	1.2	45.				
318							
#4	317:	3.9	94.1				
Wet Wi							
# 4 Wt	7654.4	94.					
Total Wt Dry	91319					_	
WA:		DATION AF GM DRY SA		HING			
Screen Size	Weight Retained	Percent Retained	Percent - Passing	Total % Passing	SPECS.	MOISTURE DET	ERMINATIO
#8	176.3	9.5	91.5	18.3			-#4
# 1O	37.6	2.5	<i>ଝ</i> ୍ବର	16.8		Container & Wet Soil Weight (gm.)	
≠ 16			1			Container & Dry	
<i>F</i> 10	121.7	8.2	තාදි	16.3		Soil Weight (gm.)	
# 20	121.7	8.2 5.4	80.8 75.5	16.3		H ₂ O Loss	
						H ₂ O Loss % Moisture	
# 20	79.7 60.4	5.4	75.5	14.8		H ₂ O Loss	0
≠20 ≠30	79.7	5.4 4.1	75.5 71.4	14.8		H ₂ O Loss Moisture A.A.S.H.	0
# 20 # 30 # 40	79.7 60.4 67.1	5.4 4.1 4.5	75.5 71.4 66.9	14.8 13.8 13.1		H ₂ O Loss Moisture A.A.S.H.	0
# 20 # 30 # 40 # 50	79.7 60.4 67.1 95.3	5.4 4.1 4.5 6.4	75.5 71.4 66.9 60.5	14.8 13.8 13.1 12.3		H ₂ O Loss Moisture A.A.S.H.	0
#20 #30 #40 #50	79.7 60.4 67.1 95.5 738.2	5.4 4.1 4.5 6.4 22.:	75.5 71.4 66.9 60.5 33.4	14.8 13.8 13.1 12.3		H ₂ O Loss Moisture A.A.S.H.	0

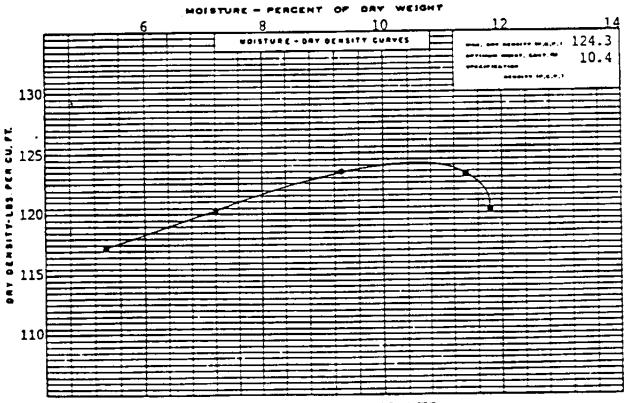
CHEN AND ASSOCIATES

Consulting Engineers
Soil and Foundation Engineering



GRADATION TEST RESULTS

SHAVEL 25 % SAND 37 % SILT AND CLAY 38 %



COMPACTION TEST RESULTS

compaction test procedure ASTM D-698 Method C

mamete of Clayey Sand and Gravel

#525284

FROM

DEPTH

UNEN AND ASSUCIATES

Consulting Soil and Foundation Engineers

TEST NUMBER	1	2	3	4		¹[\downarrow	\downarrow	lacksquare	T	1
LOCATION											13	,		\	
HEIGHT-INCH	.75	.75	.75		1	3	1	\downarrow	-	+		+	 	+	
DIAMETER-INCH	1.925	1.925	1.925		ks.		//			1	\blacksquare	4	2	lacksquare	\Box
WATER CONTENT - \$	10.0	10.0 -	10.0		,	H		+			+	+	-	+	
DRY DEHSITY - pcf	117.8	117.8	117.8		Stress	2						1			
CONSOL. LOAD - ksf	2.0	4.0	6.0							#		+			
HORMAL LOAD - ksf	2.0	4.0	6.0		Shear	\mathbb{H}				-		-	-	-	
SHEAR STRESS - ksf	1.8	3.0	4.0		}	. Ш					_				
TYPE OF SPECIMEN SOIL DESCRIPTION	Remold	ed @ 95	% Std.P			1									
JOIL DESCRIPTION				· · · · · · · · · · · · · · · · · · ·	-	-		+		-	\dashv	+	+		
									0.1			0.3	_		0
TYPE OF TEST	Consol Satura		Undraine	ed	- -	0 0 T#	IN Ø	Hor	izon (in				me	n t	
TYPE OF TEST	-		Undraine	ed	• • •		IN Ø	Hor	izon					n t	
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TYPE OF TEST	Satura	ted			- - -	TA	ø		izon (in	che		ace 29	•	n t	
	Satura	ted				TA	ø		izon (in	che		ace 29	•	n t	
	Satura	ted			-	TA	ø		izon (in	che		ace 29	•		
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6	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•		-
6	Satura	ted				TA	ø		izon (in	che		ace 29	•		
Stress - ksf	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•		· · · · · · · · · · · · · · · · · · ·
Stress - ksf	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•		· · · · · · · · · · · · · · · · · · ·
Stress - ksf	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•		
Stress - ksf	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•		-
Stress - ksf	Satura	ted		n/sec		TA	ø		izon (in	che		ace 29	•	n t	

Normal Stress + ksf

SLOPE STABILITY ANALYSIS

for

CO-OP MINE - SLOPE STABILITY ** SEDIMENT POND A

DATA FILE: "CPSLOA:T14"

PROJECT NUMBER: 8309-42

by: EGN

```
WATER UNIT WEIGHT= 62.40
                                                         Interior Slope
          X-ORD
                      Y-ORD
 POINT
                      87.00
           0.00
   1
         100.00
                      87.00
   2
                      95.00
   3
         116.00
         118.00
                      96.00
   4
   5
         127.00
                      96.00
         156.88
                      81.50
         200.00
                      81.50
                                 SOIL
      LINE
               LEFT
                       RIGHT
        1
                 1
                          2
                                    2
                          3
                                    2
        2
                 2
                                    2
        3
                 3
                          6
                 6
                          7
                                    2
        4
        5
                 3
                          4
                                    1
                          5
        6
                 4
                                    1
                 5
                                    1
                                                        SATURATED
                                  COHESION
SOIL
             UNIT WEIGHT
                                                 Ø
                                                            ИO
                                     700
                                                 29
                 118
  1
                                                           YE$
                                     700
                                                 29
  2
                 118
                                       FACTOR OF SAFETY
          X-ORD
                     Y-ORD
                              RADIUS
CIRCLE
                                19.0
                                             4.57
          108.0
                      94.0
                                                           EFF WEIGHT
                                                                                    Χ
                                                 HIDIH
                                                                           Ø
                                    COHESION
SLICE
         WEIGHT
                    INCLINATION
                                                                           29
                                                                                  101.4
                                                                120.3
                                       700
                                                   1.7
                      -41.3
            255.8
  1
                                                                                  103.2
                                                                315.4
                                                                           29
                                       700
                                                   1.7
  2
            670.6
                      -29.1
                                                                           29
                                                   1.7
                                                                467.8
                                                                                  164.9
                                      700
  3
            994.6
                      -18.3
                                                                           29
                                                                                  106.6
                                                   1.7
                                                                587.2
   4
           1248.6
                       -8.2
                                       700
                                                                           29
                                                   1.7
                                                                677.7
                                                                                  108.3
                                       700
   5
                        1.7
           1441.1
                                                                740.1
                                                                           29
                                                                                   110.0
                                                   1.7
   6
           1573.8
                       11.6
                                       700
                                                                772.6
                                                                           29
                                                                                   111.7
                                                   1.7
                       21.9
                                       700
   7
           1642.9
                                                   1.7
                                                                769.8
                                                                           29
                                                                                   113.4
                                       700
   8
           1636.9
                       33.1
                                                                            29
                                                                                   115.1
                       46.0
                                       700
                                                   1.7
                                                                718.3
   9
           1527.4
                                                                                   116.5
                                                    . 9
                                                                352.0
                                                                            29
                                       700
            702.7
                       58.1
  10
                                                                326.0
                                                                            23
                                                                                  117.4
                                                    . 9
                                       790
            555.8
                       71.0
  11
                                                                                   117.9
                                                                 50.6
                                                                            29
             69.9
                                       700
                                                    . 2
                       83.6
  12
                                           CALCULATED
                        INITIAL
 ITERATION
                                                4.3859
                         1.0000
      1
                                                4.5570
      2
                         4.3859
                                                4.5669
      3
                         4.5570
```

4.5674

94 R=

10

108 Y=

4.5669

4.57

FACTOR OF SAFETY=

EARTHQUAKE=

AT X=

W// Lik 0		021,0					
POINT	X-ORD	Y-ORD		•			
	0.00	87.00					
1					-		
2	100.00	87.00			_		
3	116.00	95.00			•		
4	118.00	96.00					
5	127.00	9£.00					
6	156.0 0	81.50					
7	200.00	81.50					
LI	NE LEFT	r RIGHT	SOIL				
	1 1	2	2				
	2 2	3	2				
	3 3	6	2				
	4 6	7	2				
		4					
			1				
	6 4	5	1				
	7 5	6	1				
SOIL	UHIT 4	4EIGHT	COHESION	ø s	ATURATED		
1	1 :	18	700	29	НО		
2	1 :	18	700	29	YES		
CIRCLE	X-ORD	Y-ORD RA	DIUS FACTOR	OF SAFET	Υ		
	110.0	98.0	10.0 5	.71			
SLICE	WEIGHT	INCLINATION	COHESION	HTCIW	EFF WEIGHT	Ø	×
1	119.8	-26.5	700	1.4	56.3	29	105.5
2	327.1	-17.9	` 700	1.4	153.8	29	106.9
3	497.2	-9.6	700	1.4	233.8	29	108.3
4	633.8	-1.6	700	1.4	298.1	29	109.7
5	738.4	6.5	700	1.4	347.2	29	111.1
6	810.3	14.6	700	1.4	381.1	29	112.5
7	846.8	23.1	700	1.4	398.2	29	113.9
		-			396.0	29	115.3
8	842.8	32.1	700	1.4			
9	571.3	40.6	700	1.0	294.8	29	116.5
10	514.1	48.8	700	1.0	320.2	29	117.5
11	423.2	59.6	700	1.1	331.7	29	118.6
12	94.5	72.3	780	.7	94.5	29	119.5
		•	**.	A.I. A.Z.Z.			
ITERATI	ΠŪΝ	INITIAL	CAL	CULATED			
1		1.0000		4.9462			
2		4.9462		5. 6651			
3		5.6651		5.7085			
		E 700E		E 7100			

5.7108

10

5.71 AT X= 110 Y= 98 R=

5.7085

FACTOR OF SAFETY=

EARTHQUAKE= .10

	1 2	1 2		2 3
LI	HE	LEFT	RIG	
7	200	.00	81.5	0
6	156	.00	81.5	0
5	127	.00	96.0	0
4	118	.00	96.0	0
3	116	.00	95.0	3
2	100	.00	87.0	0
1	8	.00	87.8	0
POINT	X-6	ORD	Y-OR	Þ

LIHE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	3	2
3	3	6	2
4	6	7	2
5	3	- 4	1
6	4	5	1
7	5	6	1

SOIL	UNIT WEIGHT	COHESION	Ø	SHIURHIED
1	118	700	29	МО
2	118	700	29	YES
_				

CIRCLE	X-ORD	Y-ORD	RADIUS	FACTOR OF	SAFETY
	104.0	98.0	10.0	10.14	

SLICE	WEIGHT	INCLINATION	COHESION	HTCIW	EFF WEIGHT	Ø	×
1	35.5	-7.0	700	1.0	16.7	29	102.8
2	98.5	-1.5	700	1.0	46.3	29	103.7
3	150.9	4.1	700	1.0	71.0	29	104.7
4	192.6	9.6	700	1.0	90.6	29	105.7
5	223.2	15.3	700	1.0	105.0	29	106.6
6	242.8	21.1	700	1.9	113.8	29	107.6
7	247.7	27.2	700	1.0	116.5	29	108.6
8	238.3	33.6	700	1.0	112.1	29	109.5
9	210.4	40.6	700	1.0	99.0	29	110.5
10	158.0	48.4	700	1.0	74.3	29	111.5
11	68.2	57.8	700	1.0	32.1	29	112.4

ITERATION	INITIAL	CALCULATED
1	1.0000	8.2214
2	8.2214	10.0653
3	10.0653	10.1340
4	10.1340	10.1361

FACTOR OF SAFETY= 10.14 AT X= 104 Y= 98 R= 10 EARTHQUAKE= .10

POINT X-OF	RU	Y-QRD		
1	0.	88	87.99	
2	100.	90	87.00	
3	116.	88	95.00	
4	118.	99	96.00	
5	127.	80	96.00	
6	156.	80	81.50	
7			81.50	
LI	I NE	LEFT	RIGHT	SOIL
	1	1	2	2
	2	2	3	2
	3	3	6	2
	4	6	7	2
	5	3	4	1
	6	4	5	1
	7	5	6	1

SOIL	UNIT WEIGHT	COHESION	Ø	SRTURATED
1	118	700	29	но
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 109.0 98.0 15.0 3.94

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH '	EFF WEIGHT	Ø	×
1	74.3	-39.9	700	1.2	34.9	29	99.4
2	632.5	-31.7	789	2.3	297.4	29	101.1
3	1249.7	-21.9	700	2.3	587.7	29	103.4
4	1748.2	-12.7	700	2.3	822.2	29	105.7
. 5	2145.1	-3.8	700	2.3	1008.8	29	108.0
6	2446.9	4.9	700	2.3	1150.8	29	110.3
7	2653.4	13.8	700	2.3	1247.8	29	112.6
8	2756.6	23.1	700	2.3	1296.4	29	114.9
9	2409.4	32.3	700	2.0	1233.4	29	117.0
10	2639.8	43.5	700	2.5	1569.2	29	119.3
11	1738.0	59.6	700	2.5	1277.1	29	121.8
12	209.5	75.7	700	. 9	209.5	29	123.4

ITERATION	INITIAL	CALCULATED
1	1.0000	3.6937
2	3.6937	3.9291
3	3.9291	3.9432
4	3.9432	3.9440

FACTOR OF SAFETY= 3.94 AT X= 109 Y= 98 R= 15 EARTHQUAKE= .10

THI	X-URU	Y-UKU
1	0.08	87.00
2	100.00	87.00
3	116.00	95.00
4	118.00	96.00
5	127.00	96.00
6	156.00	81.50
7	200 00	01 50

LINE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	3	2
3	3	6	2
4	6	7	2
5	3	4	1
6	4	5	1
7	5	6	1

SOIL	UNIT WEIGHT	COHESION	Ø	SATURATED
1	118	700	29	но
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 106.0 94.0 15.0 4.09

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
1	549.4	- 54.2	780	2.4	258.4	29	93.9
2	1283.4	-40.2	788	2.4	603.6	29	96.4
3	1755.6	-28.9	700	2.4	825.7	29	98.8
4	2488.6	-18.2	700	2.7	1170.4	29	101.3
5	3099.3	-7.7	700	2.7	1457.5	29	104.0
6	3555.5	2.6	700	2.7	1672.1	29	106.7
7	3861.2	12.9	780	2.7	1815.9	29	109.3
8	4004.5	23.7	790	2.7	1883.3	29	112.0
9	3950.6	35.5	700	2.7	1857.9	29	114.7
10	2756.1	47.5	780	2.0	1400.7	29	117.0
11	1680.6	58.4	700	1.4	961.8	29	118.7
12	1178.1	73.0	700	1.4	768.7	29	120.1
13	48.8	84.9	700	. 1	42.6	29	120.9

ITERATION	INITIAL	CALCULATED
1	1.0000	5.0280
2	5.0280	4.1140
3	4.1140	4.0898
4	4.0898	4.0892

FACTOR OF SAFETY= 4.09 AT X= 106 Y= 94 R= 15 EARTHQUAKE= .10

POINT 1 2 3 4 5 6 7	X-ORP 0.00 100.00 116.00 118.00 127.00 156.00 200.00	Y-ORD 87.00 87.00 95.00 96.00 96.00 81.50					
LI	NE LEFT 1 1 2 2 3 3 4 6 5 3 6 4 7 5	RIGHT 2 3 6 7 4 5 6	SOIL 2 2 2 2 2 1 1				
SOIL 1 2	4 TINU 11 11	8	COHESION 700 700	ø S 29 29	ATURATED NO Yes		
CIRCLE	X-ORD 105.0			OF SAFET	Y		
SLICE 1 2 3 4 5 6 7 8 9 10 11 12 13 14	WEIGHT 3.3 182.2 450.4 672.6 850.5 983.6 1069.7 1103.5 1076.2 970.8 457.1 358.3 93.8 89.2	INCLINATION -20.3 -15.9 -9.0 -2.1 4.7 11.6 18.6 26.0 33.8 42.6 50.1 56.6 61.6 66.8	COHESION 700 700 700 700 700 700 700 700 700 70	WIDTH	EFF WEIGHT 1.5 85.7 211.8 316.3 400.0 462.6 503.0 519.0 506.1 456.5 241.1 246.9 84.3 89.2	Ø 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	X 99.8 100.9 102.7 104.4 106.2 109.8 111.6 113.3 115.1 116.5 117.5 118.8
`ITERAT! 1 2 3 4	10H	INITIAL 1.0000 4.1830 4.8519 4.8964	CAL	.CULATED 4.1830 4.8519 4.8964 4.8990			

FACTOR OF SAFETY= 4.90 AT X= 105 Y= 101 R= 15

EARTHQUAKE= .10

POINT	×-	ORD	Y-ORD
1	0	.00	87.00
2	100	.00	87.00
3	116	.00	95.00
4	118	.80	96.00
5	127	. 98	96.00
á	156	.00	81.50
7	200	.00	81.50
LI	HE	LEFT	RIGHT
	1	1	2
	_	_	_

LIHE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	3	2
3	3	6	2
4	6	7	2
5	3	4	1
6	4	5	1
7	5	6	i

SOIL	UNIT WEIGHT	COHESION	Ø	SATURATED
1	118	700	29	HO
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 106.0 102.0 15.0 5.22

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
1	153.5	-14.2	700	1.8	72.2	29	102.3
2	422.8	-7.1	700	1.8	198.5	29	104.2
3	641.7	i	700	1.8	301.8	29	106.0
4	813.8	6.9	700	1.8	382.7	29	107.8
5	937.2	14.0	700	1.8	440.8	29	109.6
6	1008.4	21.3	700	1.8	474.2	29	111.4
7	1020.4	29.1	700	1.8	479.9	29	113.3
8	960.7	37.4	700	1.8	451.8	29	115.1
9	466.7	44.5	700	1.0	245.6	29	116.5
10	398.2	50.1	700	1.0	265.7	29	117.5
11	215.7	55.6	700	.7	184.1	29	118.4
12	124.6	62.3	700	1.0	124.6	29	119.2

ITERATION	INITIAL	CALCULATED
1	1.0000	4.4326
2	4.4326	5,1679
3	5.1679	5.2127
4	5.2127	5.2151

FACTOR OF SAFETY= 5.22 AT X= 106 Y= 102 R= 15 EARTHQUAKE= .10

	IT WEIGHT:	e 62 40	•		Interior	Slop	د
MHIEK ON	II MEIRUI.	- 62.40				ı	
POINT	X-ORD	Y-ORD					
1	8.00	87.00					
	100.00	87.00					
	116.00	95.80					•
	118.00	96.00					
	127.00	96.00					
	156.00	81.50					
7	200.00	81.50					
LIN	E LEFT	RIGHT	SOIL				
		2	2				
1 2		3	2				
	2	6	2				
3		7	2				
4		4	ī				
é	, 3	5	i				
7		6	1				
•	3	J	•				
SOIL	UHIT W	FIGHT	COHESION	Ø	SATURATED		
1	11		700	29	но		
2	11		700	29	YES		
-	-	. =			•		
CIRCLE	X-ORD	Y-ORD F		R OF SAFE	TY.		
•••	109.0	100.0	20.0	3.78			
SLICE	WEIGHT	INCLINATIO	N COHESION	WIDTH	EFF WEIGHT	Ø	X
1	591.0	-43.3	700	3.1	277.9	29	95.4
2	1457.2	-32.0	700	3.1	685.3	29	98.5
3	2405.2	-21.8	788	3.2	1131.2	29	101.6
4	3375.3	-12.2	700	3.2	1587.4	29	104.8
5	4137.1	-2.9	790	3.2	1945.6	29	108.0
6	4783.9	6.3	700	3.2	2212.2	29	111.2
7	5072.8	15.7	700	3.2	2385.7	29	114.4
8	3258.4	23.6	700	2.0	1636.9	29	117.0
9	4602.0	31.8	700	3.0	2572.6	29	119.5
10	3801.3	42.7	700	3.0	2385.6	29	122.5
11	2580.8	56.4	700	3.0	2001.1	29	125.5
12	1.2	64.2	700	. 8	1.2	29	127.0
13	453.1	70.3	700	1.4	453.1	29	127.7
	0 11	INITIO		RI CUL ATED			

CALCULATED ITERATION INITIAL 3.7806 3.7785 1.0000 1 3.7806 2

3.78 AT X= 109 Y= 100 R= 20 FACTOR OF SAFETY= EARTHQUAKE= .10

MATER UN	II MEIGHT	62.40					
		Y-ORD					
POINT	X-ORD						
1	8.30	87.00					
2	100.00	87.00					
3	116.00	95.00					
4	118.00	96.00	•				
5	127.00	96.00					
6	156.00	81.50					
7	200.00	81.50			•		
LIF	IE LEFT		SOIL				
		2	2				
	2 2	3	2 2 2				
	3 3	6	2				
	4 6	6 7	2				
	5 3	4	1				
Ì		5	i				
	6 4 7 5	6	1				
	7	J	•				
0071	UNIT W	ETCHT	COHESION	ø Si	ATURATED		
SOIL	11		700	29	ИО		
1	11		700	29	YES		
2	1 1	. 0		_			
	X-ORD	Y-ORD RAI	IUS FACTOR	OF SAFET	Υ		
CIRCLE	106.0			. 41			
	106.0	100.0					
	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
SLICE		-17.8	700	.2	.5	29	99.9
1	1.1	-14.1	700	2.3	123.2	29	101.1
2	261.9	-7.4	700	2.3	322.7	29	103.4
3	686.1	8	700	2.3	488.2	29	105.7
4	1038.0		700	2.3	620.4	29	108.0
5	1319.3	5.7	700	2.3	719.8	29	110.3
6	1528.9	12.4		2.3	782.0	29	112.6
7	1662.7	19.2	700	2.3	805.4	29	114.9
8	1712.6	26.3	700		791.8	29	117.0
9	1461.5	33.4	700	2.0	701.8	29	118.8
16	1055.6	40.0	700	1.7	614.1	29	120.5
11	742.9	46.7	700	1.7		29	122.3
12	353.9	55.1	700	2.0	353.9	2.7	
				A.U. ATER			
ITERAT	IOH	INITIAL	CHL	CULATED			
		1.9999		3.8643			
2		3.8643		4.3791			
1 2 3 4		4.3791		4.4085			
4		4.4085		4.4100			

FACTOR OF SAFETY= 4.41 AT X= 106 Y= 106 R= 20 EARTHQUAKE= .10

POINT	X-01	₹D	Y-ORD
1	0.6	30	87.00
2	100.6	90	87.00
3	116.0	30	95.00
4	118.0	30	96.00
5	127.8	36	96.00
6	156.6	30	81.50
7	200.8	90	81.50
1 1	t NIE	1 557	DICUT

LINE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	3	2
3	3	6	2
4	6	7	2
5	3	4	1
6	4	5	1
7	5	6	1

SOIL	. UNIT WEIGHT	COHESION	Ø	SATURATED
1	118	700	29	Ю
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 102.0 104.0 20.0 4.95

SLICE	WEIGHT	INCLINATION	COHESION	HIDTH	EFF WEIGHT	Ø	×
1	269.2	-27.2	700	2.8	126.6	29	92.9
2	667.8	-18.3	700	2.8	314.1	29	95. 7
3	906.6	-9.9	700	2.8	426.4	29	98.6
4	1148.3	-1.9	700	2.7	540.0	29	101.3
5	1539.2	5.8	700	2.7	723.9	29	104.0
6	1816.1	13.5	700	2.7	854.1	29	106.7
7	1978.8	21.6	700	2.7	926.8	29	109.3
8	1985.5	30.1	788	2.7	9 33.7	29	112.0
9	1825.4	39.5	700	2.7	858.5	29	114.7
10	1114.1	48.8	700	2.8	628.5	29	117.0
11	462.2	56.4	700	1.3	366.0	29	118.6
12	141.8	63.0	700	1.1	141.8	29	119.8

ITERATION	INITIAL	CALCULATED
1	1.0000	4.5173
2	4.5173	4.9325
3	4.9325	4.9501
4	4.9501	4.9508

FACTOR OF SAFETY= 4.95 AT X= 102 Y= 104 R= 20 EARTHQUAKE= .10

AIIIIER C	MEIGH	- 02.40					
POINT	X-ORD	Y-ORD	•				
1	0.00	87.00					
2	100.00	87.00		•			
3	116.00	95.00					
4	118.00	96.00					
5	127.00	96.00					
6	156.00	81.50					
7	200.00	81.50					
•							
LI	THE LEFT		SOIL				
	1 1	2	2				
	2 2	3	2				
	3 3	6	2				
	4 6	7	2				
	5 3 6 4	4	1				
	6 4	5	1				
	7 5	6	1				
SOIL	UHIT I	WEIGHT	COHESION	ø s	ATURATED		
1	1	18	700	29	NO		
2	1	18	700	29	YES		-
CIRCLE			ADIUS FACTOR		Y		
•	102.0	107.0	20.0 6	.40			
SLICE	WEIGHT	INCLINATION	N COHESION	HTCIW	EFF WEIGHT	Ø	×
1	103.9	-2.7	700	1.8	48.8	29	101.0
2	287.4	2.3	790	1.8	135.2	29	102.8
3	438.8	7.4	700	1.8	206.4	29	104.6
4	557.3	12.5	700	1.8	262.1	29	106.3
5	641.2	17.7	700	1.8	301.5	29	108.1
6	687.9	23.1	700	1.8	323.5	29	109.8
7	693.2	28.7	700	1.8	326.0	29	111.6
8	650.7	34.7	700	1.8	306.0	29	113.4
9	550.2	41.1	700	1.8	258.7	29	115.1
10	346.6	47.8	700	1.6	228.6	29	116.8
11	57.2	52.2	700	. 4	57.2	29	117.8
12	42.8	54.9	700	.7	42.8	29	118.4
			.				
ITERAT:	ION	INITIAL	CAL	CULATED			

 ITERATION
 INITIAL
 CALCULATED

 1
 1.0000
 5.2973

 2
 5.2973
 6.3473

 3
 6.3473
 6.4019

 4
 6.4019
 6.4043

FACTOR OF SAFETY= 6.40 AT X= 102 Y= 107 R= 20 EARTHQUAKE= .10

4.6738

86 R=

10

53 Y=

4.6741

4.67 AT X=

FACTOR OF SAFETY=

EARTHQUAKE= .10

FOINT 1 2 3 4 5 6 7	X-ORD 0.00 44.00 73.00 82.00 84.00 100.00 200.00	Y-ORD 81.50 81.50 96.00 96.00 95.00 87.00 87.00			•		
L	INE LEFT 1	RIGHT 2 5 6 7 3 4 5	SOIL 2 2 2 2 1 1				
SOIL 1 2	UNIT 11 11	8	COHESION 700 700	ø 9 29 29	ATURATED NO YES		
CIRCLE	X-0RD 50.0			OF SAFET	·Y		
SLICE 1 2 3 4 5 6 7 8 9 10 11	WEIGHT 15.8 143.2 279.5 373.8 453.8 509.8 539.8 540.1 503.4 415.0 162.0 72.6	INCLINATION -19.6 -14.2 -6.9 .3 7.6 14.9 22.5 30.6 39.4 49.7 59.2 68.8	COHESION 700 700 700 700 700 700 700 700 700 70	WIDTH	EFF WEIGHT 15.8 112.5 188.3 252.9 306.5 348.8 378.9 395.0 393.7 368.1 162.0 72.6	ø 29 29 29 29 29 29 29 29 29	X 46.5 47.8 48.1 51.3 52.8 55.1 56.6 57.6 59.5 59.3
ITERAT 1 2 3 4	ION	INITIAL 1.0000 6.0224 7.2216 7.2876	CAL	CULATED 6.0224 7.2216 7.2876 7.2907			

92 R= 10

FACTOR OF SAFETY= 7.29 AT X= 50 Y=

EARTHQUAKE= .10

POINT	X-ORD	Y-ORD					
	0.00	81.50		•			
1							
2	44.00	81.50					
3	73.00	96.00					
4	82.00	96.00					
5	84.00	95.00		•			
6	100.00	87.00					
7	200.00	87.00					
L	INE LEFT		SOIL				
	1 1	2	2				
	2 2	5	2				
	3 5	6	2				
	4 6	7	2				
	5 2	3	1				
	6 3	4	1				
	7 4	5	1			•	
COTI	W TINU	CICUT	COHESION	ø S	ATURATED		
SOIL							
1	11		700 7 00	29	NO NO		
2	1 1	. 8	700	29	YES		
CIRCLE	X-ORD	Y-ORD RAI	DIUS FACTOR	OF SAFET	Y		
O I KOLL	52.0			.24	•		
	02.0	,,,,,	,	• • •			
SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
1	36.9	-11.2	700	. 9	36.9	29	50.1
2	103.3	-5.8	700	. 9	103.3	29	51.0
3	174.9	3	700	1.0	163.3	29	52.0
4	228.9	5.5	700	1.9	198.9	29	53.0
5 6	270.8	11.3	700	1.0	228.9	29	54.0
6	299.9	17.3	700	1.0	252.8	29	55.0
7	315.3	23.5	700	1.0	270.3	29	56.0
8	315.1	29.9	700	1.0	280.5	29	57.0
9	296.4	36.8	700	1.0	281.9	29	58.0
10	232.0	44.1	700	.9	232.0	29	58.9
11	173.4	51.9	700	.9	173.4	29	59.8
12	75.7	61.6	700	. 9	75.7	29	60.8
		01.0			, 5. (00.0
		-					
ITERAT	ION	INITIAL	CAL	CULATED			
1		1.0000		7.5257			
1 2 3		7.5257		9.1670	•		
3		9.1670		9.2357			
4		9.2357		9.2381			
i							

FACTOR OF SAFETY= 9.24 AT X= 52 Y= 94 R= 10 EARTHQUAKE= .10

WATER L	HIT WEIGHT	= 62.40	•					
POINT	X-ORD	Y-ORD						
1	0.80	81.50						
2	44.00	81.50						
3	73.00	96.00						
4	82.00	96.00						
	84.00	95.00						
5 6	100.00	87.00						
7	200.00	87.00						
L	INE LEFT		SOIL					
	1 1	2	2					
	2 2	5	2					
	3 5	6	2					
	4 6	7	2					
	5 2	3	1					
	6 3	4	1					
	7 4	5	1					
SOIL	4 TINU		COHE			ATURATED		
1		18		00	29	NO NO		
2	_ 11	8	7	99	29	YES		
CIRCLE	X-ORD	Y-ORD	RADIUS		OF SAFET	Y		
	46.0	92.0	10.0	13	.12			
	WEIGHT	INCLINAT	נטא בט	HESION	WIDTH	eff weight	ø	×
SLICE	6.4	-4.0		700	. 4	6.4	29	45.3
1 2	48.3	3		780	.9	39.8	29	45.9
3	89.1	4.7		788	.9	66.6	29	46.8
4	122.1	9.7		700	. 9	89.7	29	47.7
5	147.1	14.8		700	. 9	109.0	29	48.5
6	163.6	20.0		700	. 9	124.4	29	49.4
7	171.0	25.3		700	. 9	135.5	29	50.3
8	168.0	31.0		700	. 9	141.6	29	51.1
9	152.7	37.0		700	.9	142.1	29	52.0
10	101.1	42.8		700	.7	101.1	29	52.8
11	71.7	48.5		700	. 7	71.7	29	53.5 54.2
12	28.7	54.9		700	.7	28.7	29	J7.4

ITERATION	INITIAL	CHLCOTHIED
1	1.0000	10.5338
2	10.5338	13.0454
3	13.0454	13.1176
3	13.1176	13.1192

FACTOR OF SAFETY# 13.12 AT X= 46 Y= 92 R= 10 EARTHQUAKE= .10

FACTOR OF SAFETY= 3.50 AT X= 57 Y= 90 R= 15 EARTHQUAKE= .10

WATER UNIT WEIGHT= 62.40 X-ORD POINT Y-ORD 0.00 1 81.59 2 44.00 81.50 3 73.99 96.00 4 82.00 96.00 5 84.00 95.00 6 100.00 87.00 200.00 87.00 LINE LEFT RIGHT SOIL 1 1 2 2 2 2 5 2 3 5 6 2 4 6 7 5 2 3 1 6 3 UNIT WEIGHT SOIL COHESION Ø SATURATED 1 118 700 29 ИΘ 2 118 700 29 YES CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 58.0 98.0 15.0 4.45 SLICE WEIGHT INCLINATION COHESION WIDTH EFF WEIGHT Ø X 1 105.4 -26.1 700 1.3 105.4 29 51.4 2 558.7 -19.0 799 2.1 458.1 29 53.1 3 959.9 -10.7 700 2.1 691.9 29 55.2 1282.8 -2.5 700 2.1 889.0 29 57.3 5 1531.3 5.6 700 2.1 1051.0 29 59.5 6 1784.3 13.8 700 2.1 1177.6 29 61.6 2.1 7 1796.2 22.3 700 1266.0 29 63.7 8 1793.9 31.4 700 2.1 1310.1 29 65.8 9 1670.8 41.5 788 2.1 67.9 1297.4 29 10 1365.3 53.8 700 2.1 1198.9 29 70.0 11 624.6 71.3 700 1.8 624.6 29 72.0

ITERATION	INITIAL	CALCULATED
1	1.0000	3.8124
2	3.8124	4.4023
3	4.4023	4.4514
4	4.4514	4.4550

FRCTOR OF SAFETY= 4.45 AT X= 58 Y= 98 R= 15 EARTHQUAKE= .10

POINT	X-ORD	Y-ORD
1	9.00	81.50
2	44.80	81.50
3	73.00	96.00
4	82.90	96.00
5	84.90	95.00
6	100.00	87.00
7	200.00	87.00

LINE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	. 5	2
3	5	6	2
4	6	7	2
5	2	3	1
6	3	4	1
7	4	5	1

SOIL	UNIT WEIGHT	COHESION	Ø	SATURATED
1	118	700	29	NO
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 52.0 96.0 15.0 4.70

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	X
1	7.3	-23.9	700	. 4	7.3	29	45.9
2	264.1	-19.3	700	1.9	182.7	29	47.0
3	593.0	-11.8	700	1.9	373.9	29	48.9
4	864.7	-4.5	700	1.9	538.1	29	50.8
5	1082.3	2.8	700	1.9	676.8	29	52.7
6	1246.8	10.1	700	1.9	790.2	29	54.6
7	1353.5	17.6	700	1.9	877.2	29	56.5
8	1398.9	25.4	790	1.9	934.9	29	58.4
9	1371.1	33.8	700	1.9	958.3	29	60.3
10	1248.9	43.1	700	1.9	937.2	29	62.2
1 1	985.1	54.3	700	1.9	849.5	29	64.1
12	412.6	69.1	700	1.6	412.6	29	65.9

ITERATION	INITIAL	CALCULATED
i	1.0000	3.9771
2	3.9771	4.6404
3	4.6404	4.6925
4	4.6925	4.6961

FACTOR OF SAFETY= 4.70 AT X= 52 Y= 96 R= 15 EARTHQUAKE= .10

FACTOR OF SAFETY= EARTHQUAKE= .10

						•	
POINT	X-ORD	Y-ORD					
	9.00	81.50			•		
1	44.80	81.50					
2		96.00					
3	73.00		•				
4	82.00	96.00					
5	84.00	95.00					
6	100.00	87.00					
7	220.00	87.00					
L	INE LEFT	RIGHT S	BOIL				
	1 1	2	2				
	2 2	5	2				
	3 5	6	2 2 2				
	4 6	7	2				
	5 2	3	1				
	6 3	4	1				
	7 4	5	i				
		IC I CUT	COHESION	ø Si	ATURATED		
SOIL	TINU		700	29	HO		
1		18	780	29	YES		
2	1.	18	יטטי	23	123		
		u 665 500	IUS FACTOR	OF SAFET	Y		
CIRCLE				.30	•		
	47.8	97.0 1	5.0 8	. 30			
							v
SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	X
1	7.2	-6.8	700	. 4	7.2	29	45.4
2	113.9	-2.4	700	1.4	88.1	29	46.4
3	233.3	3.1	700	1.4	165.0	29	47.8
4	329.4	8.6	700	1.4	231.0	29	49.2
5	401.6	14.2	700	1.4	285.8	29	50.7
6	448.4	19.9	700	1.4	328.6	29	52.1
7	467.3	25.9	700	1.4	358.3	29	53.5
8	454.3	32.2	700	1.4	373.0	29	55.0
	403.0	38.9	700	1.4	369.7	29	56.4
9	302.2	46.4	700	1.4	302.2	29	57.8
10	128.9	55.2	700	1.4	128.9	29	59.3
11	120.9	53.2					
		******	cor	CULATED			
ITERA		INITIAL	Chi	6.75 0 7			
	1	1.0000		8.2322			•
	2 3	6.7507					
		8.2322		8.2974	•		
	4	8.2974		8.2998			

8.30 AT X= 47 Y= 97 R= 15

```
WATER UNIT WEIGHT= 62.40
                                                            Exterior Slope
POINT
         X-ORD
                     Y-ORD
          0.00
                    81.50
   1
   2
         44.90
                     81.50
   3
         73.00
                     96.80
   4
         82.00
                     96.00
   5
         84.00
                     95.00
   6
        100.00
                     87.00
   7
        200.00
                     87.00
                     RIGHT
     LINE
             LEFT
                                SOIL
                         2
                                  2
       1
                1
                2
                         5
                                  2
       2
       3
                5
                         6
                                  2
                         7
                6
                                  2
       4
       5
                2
                         3
                                  1
       6
                3
                         4
                                  1
       7
                4
                                  1
           UNIT WEIGHT
                                COHESION
                                                     SATURATED
SOIL
                                              Ø
                                   760
                                              29
                118
                                                         NO
  1
  2
                                   700
                                              29
                                                         YES
                118
CIRCLE
         X-ORD
                   Y-ORD
                             RADIUS
                                     FACTOR OF SAFETY
          58.0
                     95.0
                               20.0
                                           3.00
SLICE
        WEIGHT
                  INCLINATION
                                  COHESION
                                              MIDTH
                                                         EFF WEIGHT
                                                                                X
                                                                        Ø
            35.8
                     -46.0
                                     700
                                                                        29
                                                                                43.6
                                                 .8
                                                              16.8
  1
                                                 3.2
                     -38.5
                                     700
                                                             589.3
  2
         1141.1
                                                                        29
                                                                                45.6
                                                 3.2
  3
         2540.2
                     -27.4
                                     700
                                                            1352.6
                                                                        29
                                                                                48.8
  4
                                                 3.2
         3653.0
                     -17.4
                                     700
                                                            1981.2
                                                                        29
                                                                                52.1
                                                 3.2
  5
         4537.0
                      -7.8
                                     700
                                                            2502.2
                                                                        29
                                                                                55.3
  6
         5216.8
                       1.4
                                     700
                                                 3.2
                                                            2927.2
                                                                        29
                                                                                58.5
                      10.8
  7
         5698.1
                                     700
                                                 3.2
                                                            3258.8
                                                                                61.7
                                                                        29
                                                 3.2
  8
         5970.8
                      20.4
                                     700
                                                            3491.9
                                                                        29
                                                                                64.9
                      30.7
                                     700
                                                 3.2
                                                            3611.2
  9
         5999.8
                                                                        29
                                                                                68.2
                                                 3.2
 10
          5713.3
                      42.3
                                     700
                                                            3581.8
                                                                        29
                                                                                71.4
 11
         3659.4
                      54.7
                                     700
                                                 2.4
                                                            2377.4
                                                                        29
                                                                                74.2
         2355.2
                      72.4
                                     700
                                                 2.4
                                                            1638.0
                                                                        29
                                                                                76.7
 12
           25.1
                      85.6
                                     700
                                                              25.1
                                                                        29
                                                                                77.9
 13
                                                  . 1
ITERATION
                      INITIAL
                                         CALCULATED
     1
                       1.0000
                                             2.8932
     2
                                             2.9918
                       2.8932
     3
                       2.9918
                                             2.9982
     4
                       2.9982
                                             2.9986
```

FACTOR OF SAFETY=

EARTHQUAKE=

3.00

AT X=

58 Y=

95 R=

20

	*						
POINT	X-ORD	Y-ORD					
1	0.00	81.50					
2	44.00	81.50			•		
3	73.00	96.00					
4	82.00	95.00					
	84.00	95.00					
5							
6	100.00	87.00					
7	200.00	87.00					
L	INE LEFT	T RIGHT	SOIL				
	1 1	2	2				
	2 2	5	2				
	3 5	6	2 2				
	4 6	7	2				
	5 2	3	1				
	6 3	4	i				
	7 4	5	i				
	, ,	J	•				
SOIL	UNIT	WEIGHT	COHESION	ø S	ATURATED		
1	1	18	700	29	ИŨ		
2		18	700	29	YES		
_							
CIRCLE	X-ORD	Y-ORD RA	DIUS FACTOR	OF SAFET	Y		
	50.0	100.0	20.0 4	.21			
			0011507011		EFF WEIGHT	α	×
SLICE		INCLINATION		WIDTH		ø 30	43.2
1	58.1	-19.9	700	1.6	27.3	29	
2	468.3	-13.7	700	2.5	252.9	29	45.3
3	981.2	-6.3	700	2.5	559.4	29	47.8
4	1395.7	1.0	700	2.5	819.6	29	50.3
5	1713.5	8.3	700	2.5	1034.4	29	52.9
6	1931.5	15.8	700	2.5	1202.2	29	5 5.4
7	2040.9	23.5	700	2.5	1319.0	29	58.0
8	2024.6	31.7	700	2.5	1376.5	29	60.5
9	1849.6	40.8	700	2.5	1359.5	29	63.0
18	1446.3	51.5	700	2.5	1235.2	2 9	65.6
11	603.2	65.0	700	2.3	603.2	29	68.0
				AU 0753			
ITERA1	TION	INITIAL	CAL	CULATED			
	l	1.0000		3.6025			
2	2 3	3.6025		4.1625			
		4.1625		4.2063			
	4	4.2063		4.2093			

FACTOR OF SAFETY= 4.21 AT X= 50 Y= 100 R= 20

EARTHQUAKE= .10

700

700

1.5

1.5

581.7

250.1

29

29

68.4

69.9

ITERATION	INITIAL	CALCULATED
1	1.0000	3.8773
2	3.8773	4.5600
3	4.5600	4.6115
4	4.6115	4.6148

64.0

12

250.1

FACTOR OF SAFETY= 4.61 AT X≖ 52 Y= 102 R= 20 EARTHQUAKE= .10

WATER UNIT WEIGHT = 62.40 POINT X-ORD Y-ORD 0.00 81.50 1 2 44.00 81.50 3 73.00 96.00 4 82.00 96.00 5 84.00 95.00 6 100.00 87.00 200.00 87.00 LINE LEFT RIGHT SOIL 1 1 2 2 2 5 2 3 5 6 2 4 6 7 2 5 2 3 6 3 4 1 7 4 SOIL UNIT WEIGHT COHESION Ø SATURATED 1 118 700 29 ΝО 2 118 790 YES 29 CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 46.0 102.0 20.0 7.88 SLICE WEIGHT INCLINATION COHESION EFF WEIGHT WIDTH Ø X 1 6.6 -2.1 700 . 5 6.6 29 45.3 2 . 9 125.4 700 1.6 97.1 29 46.3 3 263.6 5.6 700 1.6 189.0 29 47.9 4 376.1 10.3 700 1.6 268.7 49.6 29 5 461.9 15.1 700 1.6 335.9 29 51.2 6 519.5 20.0 700 1.6 389.8 29 7 546.5 25.0 700 1.6 429.3 29 54.4 8 539.3 30.3 700 1.6 452.8 29 56.1 9 492.6 35.9 799 1.6 457.6 29 57.7 10 338.0 41.3 700 1.3 338.0 29 59.2 11 239.0 46.7 700 1.3 239.0 29 60.5 94.8 52.7 700 1.3 94.8 29 61.9

CALCULATED

7.88 AT X= 46 Y= 102 R=

6.4094

7.8126

7.8758

7.8782

20

ITERATION

1

2

3

FACTOR OF SAFETY=

EARTHQUAKE= .10

INITIAL

1.0000

6.4094

7.8126

7.8758

```
WATER UNIT WEIGHT= 62.40
                                                  Exterior Slope
 POINT
        X-ORD
                  Y-ORD
   1
         0.00
                81.50
   2
         44.00
                  81.50
   3
        73.00
                  96.00
   4
        82.00
                  96.00
   5
        84.00
                  95.00
                  87.00
        100.00
   7
        200.00
                   87.00
    LINE
            LEFT RIGHT
                             SOIL
       1
             1
                    2
                               2
       2
                       5
               2
                               2
                      6
       3
              5
                               2
              6
                       7
       4
       5
              2
                       3
       6
               3
               4
                               1
SOIL
          UNIT WEIGHT
                             COHESION
                                                SATURATED
                                          Ø
  1
               118
                                700
                                          29
                                                    NO
  2
               118
                                700
                                          29
                                                   YES
CIRCLE X-ORD
                 Y-ORD
                          RADIUS FACTOR OF SAFETY
                                      2.82
         57.0
                 99.0
                           25.0
        WEIGHT
SLICE
                INCLINATION
                                                   EFF WEIGHT
                               COHESION
                                          WIDTH
                                                                 Ø
                                                                        ×
                                                       155.6
         330.9
                               700
 1
                  -41.8
                                            2.4
                                                                 29
                                                                        40.4
  2
                   -34.7
         876.6
                                 700
                                            2.4
                                                       412.3
                                                                 29
                                                                        42.8
                  -26.0
  3
        2938.2
                                 700
                                            4.1
                                                      1468.8
                                                                 29
  4
                   -15.8
        4718.6
                                 700
                                            4.1
                                                      2480.1
                                                                 29
                                                                        50.2
  5
        6119.1
                   -6.1
                                 700
                                            4.1
                                                      3312.9
                                                                 29
                                                                        54.4
  6
                    3.5
        7176.4
                                 700
                                            4.1
                                                      3984.1
                                                                 29
                                                                        58.5
                                                      4495.8
  7
        7894.5
                    13.1
                                 700
                                                                        62.€
                                            4.1
                                                                 29
                                                     4835.5
4969.8
4419.6
  8
        8246.7
                    23.1
                                 700
                                            4.1
                                                                 29
                                                                        66.8
  9
        8162.1
                    34.1
                                 700
                                            4.1
                                                                 29
                                                                        70.9
                    47.0
        7138.5
 10
                                 700
                                            4.3
                                                                 29
                                                                        75.1
        4073.3
                    66.5
                                 700
 11
                                            4.3
                                                      2595.1
                                                                 29
                                                                        79.4
          35.2
                                 700
                    81.0
                                             . 3
                                                        35.2
                                                                 29
                                                                        81.7
ITERATION
                    INITIAL
                                     CALCULATED
                     1.8800
     1
                                         2.7148
     2
                     2.7148
                                         2.8136
     3
                    2.8136
                                         2.8196
                     2.8196
                                         2.8200
```

FACTOR OF SAFETY=

EARTHQUAKE= .10

2.82

AT X=

57 Y=

99 R=

25

WATER UNIT HEIGHT= 62.40 POINT X-ORD Y-ORD 1 0.00 81.50 2 44.88 81.50 3 73.00 96.00 4 82.99 96.00 5 84.00 95.00 6 100.00 87.00 200.00 87.00 LINE LEFT RIGHT SOIL 1 1 2 2 2 5 2 3 5 6 2 4 6 7 2 5 2 3 1 6 3 4 1 SOIL UNIT WEIGHT COHESION SATURATED ø 1 118 700 29 NO 2 118 700 29 YES CIRCLE RADIUS FACTOR OF SAFETY X-ORD Y-ORD 50.0 25.0 100.0 3.27

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
1	633.0	-37.1	786	3.6	297.7	29	35.0
2	1590.7	-27.2	700	3.6	748.1	29	38.6
3	2230.1	-18.2	700	3.6	1048.8	29	42.2
4	3526.1	-9. 1	700	4.1	1745.3	29	46.1
5	4688.1	. 5	700	4.1	2465.8	29	50.2
6	5512.7	10.1	700	4.1	3027.7	29	54.4
7	5983.5	- 20.0	700	4.1	3423.1	. 29	5 8.5
8	6046.2	30.5	700	4.1	3626.6	29	62.6
9	55 73.1	42.5	700	4.1	3578.2	29	66.8
10	4216.8	57.9	790	4.1	3114.0	29	70.9
11	302.4	68.5	700	. 5	284.9	29	73.2
12	376.3	75.4	700	1.2	376.3	29	74.1

ITERATION	INITIAL	CALCULATED
1	1.0000	3.0608
2	3.0608	3.2527
3	3.2527	3.2655
4	3.2655	3.2663

FACTOR OF SAFETY= 3.27 AT X= 50 Y= 100 R= 25 EARTHQUAKE= .10

10.8	-10.3	· 700	1.0	5.1	29	43.5
283.9	-6.3	700	2.5	164.9	29	45.2
692. 3	6	700	2.5	419.6	29	47.7
1028.1	5.1	700	2.5	640.2	29	50.2
1290.4	10.9	700	2.5	826.3	29	52.7
1475.9	16.7	798	2.5	976.3	29	55.2
1578.7	22.8	700	2.5	1087.3	29	57.7
1588.6	29.2	700	2.5	1154.6	29	60.2
1488.9	36.0	700	2.5	1170.4	29	62.6
1251.1	43.4	700	2.5	1121.3	29	65.1
687.7	50.9	700	1.9	687.7	29	67.4
284.9	58.7	700	1.9	284.9	29	69.3
	283.9 692.3 1028.1 1290.4 1475.9 1578.7 1588.6 1488.9 1251.1 687.7	283.9 -6.3 692.36 1028.1 5.1 1290.4 10.9 1475.9 16.7 1578.7 22.8 1588.6 29.2 1488.9 36.0 1251.1 43.4 687.7 50.9	283.9 -6.3 700 692.36 700 1028.1 5.1 700 1290.4 10.9 700 1475.9 16.7 700 1578.7 22.8 700 1588.6 29.2 700 1488.9 36.0 700 1251.1 43.4 700 687.7 50.9 700	283.9 -6.3 700 2.5 692.3 6 700 2.5 1028.1 5.1 700 2.5 1290.4 10.9 700 2.5 1475.9 16.7 700 2.5 1578.7 22.8 700 2.5 1588.6 29.2 700 2.5 1488.9 36.0 700 2.5 1251.1 43.4 700 2.5 687.7 50.9 700 1.9	283.9 -6.3 700 2.5 164.9 692.36 700 2.5 419.6 1028.1 5.1 700 2.5 640.2 1290.4 10.9 700 2.5 826.3 1475.9 16.7 700 2.5 976.3 1578.7 22.8 700 2.5 1087.3 1588.6 29.2 700 2.5 1154.6 1488.9 36.0 700 2.5 1170.4 1251.1 43.4 700 2.5 1121.3 687.7 50.9 700 1.9 687.7	283.9 -6.3 700 2.5 164.9 29 692.36 700 2.5 419.6 29 1028.1 5.1 700 2.5 640.2 29 1290.4 10.9 700 2.5 826.3 29 1475.9 16.7 700 2.5 976.3 29 1578.7 22.8 700 2.5 1087.3 29 1588.6 29.2 700 2.5 1154.6 29 1488.9 36.0 700 2.5 1170.4 29 1251.1 43.4 700 2.5 1121.3 29 687.7 50.9 700 1.9 687.7 29

ITERRTION INITIAL CALCULATED 1 1.0000 3.9636 2 3.9636 4.6680 3 4.6680 4.7174 4.7174 4.7203

FACTOR OF SAFETY= 4.72 AT X≖ 48 Y= 106 R= 25

EARTHQUAKE= .10

WATER U	HIT WEIGHT	= 52.40		•			
POINT	X-ORD	Y-ORD					
1	0.00	81.50					
2	44.00	81.50					
3	73.00	96.00			*		
4	82.00	96.88					
	84.00	95.00					
5							
6	100.00	87.00					
7	200.00	87.00					
LI	NE LEFT	r RIGHT	SOIL				
	1	2	2				
	2 2	5	2				
	3 5	6	2				
	4 6	7	2				
	5 2	3	<u> </u>				
	6 3	4	1				
	7 4	5	i				
	•	J	•				
SOIL	ו דואט	WEIGHT	COHESION	ø S	ATURATED		
1	_	18	700	29	но		
2		18	700	29	YÉS		
-	-						
CIRCLE	X-ORD	Y-ORD RE	DIUS FACTOR	OF SAFET	Y		
OINCL	42.0	106.0		.03			
	, 200	•••					
							U
SLICE	WEIGHT	INCLINATION		HIDIH	EFF WEIGHT	Ø	X
1	56.9	-8.8	700	2.3	26.7	29	38.2
2	124.8	-3.4	700	2.3	58.7	29	40.5
3	133.1	1.9	700	2.3	62.6	29	42.8
4	210.3	7.0	700	2.1	121.7	29	45.1
5	386.5	12.0	700	2.1	250.1	29	47.2
6	514.6	17.0	790	2.1	355.9	29	49.3
7	591.2	22.2	700	2.1	437.5	29	51.4
8	611.2	27.5	700	2.1	492.5	29	53.5
9	566.4	33.2	700	2.1	517.0	29	55.7
10	477.6	39.5	700	2.3	477.6	29	57.9
11	202.3	46.9	796	2.3	202.3	29	60.2
	•						
			54 :	CUI OTER			
ITERAT	ION	INITIAL	CHI	CULATED			
1		1.0000		9.4221	•		
2		9.4221		10.9930			
3		10.9930		11.0292	•		
4		11.0292		11.0299			

25

11.03 AT X= 42 Y= 106 R=

FACTOR OF SAFETY= EARTHQUAKE= .10

```
MATER UNIT MEIGHT# 62.40
                                                   Exterior Slope
                  Y-ORD
      X+ORD
 POINT
                  81.50
        0.00
   1
                  81.50
        44.88
   2
                  96.88
   3
        73.00
                  96.00
        82.00
                  95.00
   5
        84.00
        100.00
                  87.80
   6
                  87.00
        200.00
     LINE LEFT RIGHT
                          SOIL
                    2
                              2
            1
       1
                      5
                              2
              2
       2
                              2
                      6
              5
       3
                      7
                              2
              6
       4
                      3
                              1
               2
       5
                      4
       ε
               3
                                              SATURATED
                            COHESION
                                        Ø
         UNIT WEIGHT
 SOIL
                                        29
                                                 NO.
                               700
            118
  1
                                                 YES
                                         29
                               700
              118
   2
        X-ORD Y-ORD RADIUS FACTOR OF SAFETY
 CIRCLE
         55.0 102.0 30.0
                                      2.77
                                                           ø
29
                                         WIDTH
                             COHESION
                                                                     ×
                                                 EFF WEIGHT
      WEIGHT INCLINATION
 SLICE
                                                                     34.9
                                         3.6
                                                     359.1
                                788
                 -42.2
         763.6
   1
                                                                     38.5
                                                    923.5
                                                             29
                                          3.6
                                700
         1963.7
                   -33.4
   2
                                                                     42.2
                                                   1333.7
                                                             29
                                          3.6
                                700
                  -25.4
   3
         2836.0
                                                               29
                                788
                                                    2650.0
                                          4.8
                   -16.7
   4
         5382.9
                                                                     51.3
                                                               29
                                           4.8
                                                    3806.8
                                700
                    -7.2
   5
         7339.0
                                                               29
                                                                     56.1
                                                    4748.5
                                           4.8
                                700
                    2.1
   6
        _8837.8
                                                 - - 5480.0 --- 29
                                                                     60.9
                                790
                                        · 4.8
                   11.4
   7
         9889.4
                                                               29
                                                                     65.8
                                                    5988.3
                                           4.8
                                700
                    21.1
   8
        10466.6
                                                                     70.6
                                                               29
                                                    6236.9
                                           4.8
                                788
                    31.5
        10491.6
   9
                                                                     75.3
                                                             29
                                           4.5
                                                    5132.4
                    42.7
                                700
        8552.9
  10
                                                             29
                                                                     79.8
                                                    3414.4
                                           4.5
                  56. <del>-</del>
69. 7
                                788
         5806.7
  11
                                                             29
                                                                     83.8
                                          2.8
                                                     577.7
                                700
         1006.1
  12
                                                                     84.1
                                                               29
                                                       3.0
                                 700
           6.3
                    75.8
  13
                                     CALCULATED
                    INITIAL
 ITERATION
                                        2.7349
                     1.0000
      1
                                        2.7707
                     2.7349
      2
                                        2.7723
                     2.7787
```

55 Y= 102 R=

2.77 AT X=

FACTOR OF SAFETY= EARTHQUAKE= .10 30

HATER UNIT HEIGHT = 62.40

X-ORD:	Y-ORD
0.00	81.50
44.88	81.50
73.80	96.00
82.00	96.80
84.00	95.00
100.00	87.00
200.00	87.00
	0.00 44.00 73.00 82.00 84.00 100.00

LINE	LEFT	RIGHT	SOIL
1	1	2	2
2	2	5	2
3	5	6	2
4	6	7	2
5	2	3	1
6	3	4	1
7	4	5	1

SOIL	UNIT WEIGHT	COHESION	Ø	SATURATED
1	118	700	29	МО
2	118	700	29	YES

CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 50.0 100.0 30.0 3.00

SLICE	WEIGHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	Ø	×
1	1382.1	-45.9	788	4.4	612.4	29	28.6
2	3222.5	-34.7	700	4.4	1515.5	29	33.0
3	4525.5	-24.9	788	4.4	2128.3	29	37.4
4	5373.3	-15.9	700	4.4	2527.0	29	41.8
5	7113.4	-6.9	700	4.8	3463.8	29	46.4
6	8596.8	2.4	799	4.8	4398.3	29	51.3
7	9632.8	11.7	780	4.8	5122.4	29	56.1
8	10192.6	21.4	700	4.8	5622.5	29	60.9
9	10196.3	31.8	788	4.8	5861.2	29	65.8
	9468.9	43.7	700	4.8	5752.2	29	70.6
10	4878.3	55.3	780	3.1	3112.1	29	74.6
11	2775.9	68.9	700	3.1	1916.7	29	77.7
12 13	80.3	79.8	700	.5	80.3	29	79.5

ITERATIÓN	INITIAL	CALCULATED
1	1.0000	3.0347
2	3.0347	2.9976
3	2.9976	2.9960

FACTOR OF SAFETY= 3.00 AT X= 50 Y= 100 R= 30 EARTHQUAKE= .10

6	X-ORD 0.00 44.00 73.00 82.00 84.00 100.00 200.00	Y-ORD 81.50 81.50 96.00 96.00 95.00 87.00					
LIN 1 2 3 4 5 6	1 2 5 5 6 5 2 3	RIGHT 2 5 6 7 3 4	SOIL 2 2 2 2 1 1				
\$01L 1 2	UNIT W 11 11	8	COHESION 700 700	29	ATURATED NO Yes		
CIRCLE	X-ORD 45.8			OF SAFET	Y		
SLICE 1 2 3 4 5 6 7 8 9 18 11 12	WEIGHT 827.9 2069.8 2867.2 3289.8 4095.7 5828.9 5562.9 5638.5 5126.7 3713.4 614.8 67.7	INCLINATION -33.4 -23.7 -14.8 -6.2 2.5 11.3 20.4 30.1 40.9 54.2 65.3 70.8	COHESION 700 700 700 700 700 700 700 700 700 70	WIDTH 4.4 4.4 4.6 4.6 4.6 4.6 4.6 4.6	EFF WEIGHT 389.4 973.4 1348.4 1546.8 2031.9 2682.2 3144.8 3391.7 3362.5 2989.2 614.8 67.7	\$ 9 29 29 29 29 29 29 29 29 29 29	X 28.6 33.0 37.4 41.8 46.3 50.8 55.4 60.0 64.5 69.1 72.2 73.3
ITERATI 1 2 3 4	он	INITIAL 1.0000 3.5111 3.7524 3.7651	CAL	3.5111 3.7524 3.7651 3.7657		·	

FACTOR OF SHFETY= 3.77 AT X= 45 Y= 105 R= 30

EARTHQUAKE= .10

POINT 1 2 3 4 5 6	X-ORD 0.00 44.00 73.00 82.00 84.00	Y-ORD 81.50 81.50 96.00 96.00 95.00					
7	200.00	87.00					
	1 1 2 2 3 5 4 6 5 2 6 3	2 5 6 7 3 4	SOIL 2 2 2 2 2 1				
	7 4	5	1				
SOIL 1 2		EIGHT 18	COHESION 700 700	ø S: 29 29	ATURATED NO YES		
CIRCLE	X-ORD 52.0	Y-ORD RF 112.0		OF SRFET	Y		
SLICE 1	WEIGHT	INCLINATION	700	WIDTH	EFF WEIGHT	ø 29	X 46.5 48.3
) 2 3	481.0 1055.4	-7.1 -1.6	700 700	2.9 2.9	352.3 708.2	29 29	51.2
4	1532.7	4.0	789	2.9	1018.5	29 29	54.1 57.0
5 6	1912.4 2198.9	9.6 15.3	780 780	2.9 2.9	1282.8 1499.7	29 29	59.9
7	2361.4	21.2	700	2.9	1665.6	29	62.8
8	2412.2	27.3	788	2.9	1775.3	29	65.7
9 10	2324.4 2067.1	33.7 40.8	70 0 700	2.9 2.9	1819.9 1784.7	29 29	68.6 71.5
11	594.6	45.8	788	1.0	570.9	29	73.5
12		49.7	799	1.7	674.7	29	74.9
13	247.9	55.0	790	1.7	247.9	29	76.5
ITERAT	t n N	INITIAL	Ce:	CULATED	·		
1		1.8000		3.3179			
2		3.3179		3.8044			
2		3.8044		3.8394			
4		3.8394		3.8416			

FACTOR OF SAFETY= 3.84 AT X= 52 Y= 112 R= 30

EARTHQUAKE= .10

55 Y= 105 R=

35 -

FACTOR OF SAFETY=

EARTHQUAKE# .18

2.77 AT X=

WATER UNIT WEIGHT = 62.40 POINT X-ORL Y-ORD 1 0.00 81.50 2 44.88 81.50 3 73.88 96.00 4 82.00 96.00 5 84.00 95.00 6 180.00 87.00 7 200.00 87.88 LINE LEFT RIGHT SOIL 2 2 1 1 2 2 5 2 3 5 6 2 4 6 7 2 5 2 3 6 3 4 1 7 5 4 1 SOIL UNIT WEIGHT COHESION SATURATED Ø 1 118 700 29 NO 2 118 700 YES 29 CIRCLE X-ORD Y-ORD RADIUS FACTOR OF SAFETY 58.0 108.8 35.8 2.79 SLICE WEIGHT INCLINATION COHESION WIDTH EFF WEIGHT Ø X 1 919.2 -36.3 788 4.4 432.3 29 37.4 2 2358.1 -27.7 788 4.4 1109.0 29 41.8 3 4484.6 -19.4700 4.8 2189.9 29 46.4 4 6529.5 -11.1 700 4.8 3426.1 29 51.3 5 8249.6 -3.1 700 4.8 4471.9 29 56.1 6 9586.2 4.8 700 4.8 5337.4 29 60.9 7 10536.8 12.8 788 4.8 6021.3 29 65.8 8 11075.8 21.1 700 4.8 6511.3 29 70.6 9 9782.4 29.6 798 4.5 5718.6 29 75.3 18 8174.9 38.6 788 4.5 4528.1 29 79.8

788

788

2.8

5.3

1433.6

1697.1

29

29

83.6

86.7

ITERATION	INITIAL	CALCULATED
1	1.0000	2.7363
2	2.7363	2.7918
3	2.7910	2.7928

45.6

55.7

11

12

2826.8

3608.6

FACTOR OF SAFETY= 2.79 AT X= 58 Y= 108 R= 35 EARTHQUAKE= .10

```
HATER UNIT WEIGHT# 62.40
 POINT
          X-ORI
                     Y-OFD
   1
          0.00
                    81.50
   2
          44.08
                    81.50
   3
          73.00
                    96.00
          82.00
                    96.00
   5
         84.00
                    95.00
   6
         100.00
                    87.00
         200.08
                    87.00
     LINE
              LEFT
                     RIGHT
                               SOIL
       1
               1
                        2
                                 2
        2
                2
                        5
                                 2
        3
                5
                        6
                                 2
        4
                6
                        7
                                 2
       5
                2
                        3
       6
                3
                                 1
                        5
                                 1
SOIL
           UHIT WEIGHT
                               COHESION
                                                   SATURATED
                                            Ø
  1
                118
                                  700
                                            29
                                                      ΝD
  2
                118
                                  788
                                            29
                                                      YES
CIRCLE
         X-ORD
                  Y-ORD
                           RADIUS FACTOR OF SAFETY
          50.0
                  100.0
                              35.0
                                      2.93
SLICE
        WEIGHT
                  INCLINATION
                                 COHESION
                                            HIDTH
                                                      EFF WEIGHT
                                                                    Ø
                                                                            ×
 1
         2844.9
                   -50.4
                                 700
                                              5.9
                                                         1337.9
                                                                    29
                                                                            23.3
  2
         6727.7
                    -36.7
                                   700
                                              5.9
                                                         3164.8
                                                                    29
  3
                                                                            29.2
         9199.3
                    -25.3
                                   789
                                              5.9
                                                         4326.3
                                                                    29
                                                                            35.1
 4
        10706.7
                    -14.9
                                   766
                                              5.9
                                                         5835.2
                                                                    29
                                                                            41.0
  5
        12178.2
                    -5.1
                                   700
                                              5.8
                                                         5894.8
                                                                    29
                                                                           46.9
  6
        14174.3
                     4.4
                                   700
                                              5.8
                                                         7177.7
                                                                    29
                                                                           52.7
  7
        13511.0
                     14.1
                                   799
                                              5.8
                                                         8147.4
                                                                    29
                                                                           58.5
  8
        16121.3
                     24.2
                                   700
                                              5.8
                                                        8775.6
                                                                    29
                                                                           64.3
  9
        15853.2
                     35.3
                                   799
                                              5.8
                                                        8990.6
                                                                    29
                                                                           70.1
 18
        18727.7
                     46.4
                                   700
                                              4.5
                                                        6155.1
                                                                    29
                                                                           75.3
11
        7653.3
                     58.9
                                   798
                                              4.5
                                                        4282.8
                                                                    29
                                                                           79.8
12
         1687.3
                     71.2
                                   700
                                              2.8
                                                         898.1
                                                                    29
                                                                           83.0
13
         128.2
                     78.8
                                   700
                                               . 6
                                                          60.3
                                                                    29
                                                                           84.3
```

ITERATION .	INITIAL	CALCULATED
1	1.0000	3.2239
2 ,	3.22 39	2,9346
3	2.9346	2.9289
4	2.9289	2.9287

FACTOR OF SAFETY* 2.93 AT X# 50 Y* 180 R* 35 EARTHQUAKE* .10

FACTOR OF SAFETY=

EARTHQUAKE= .10

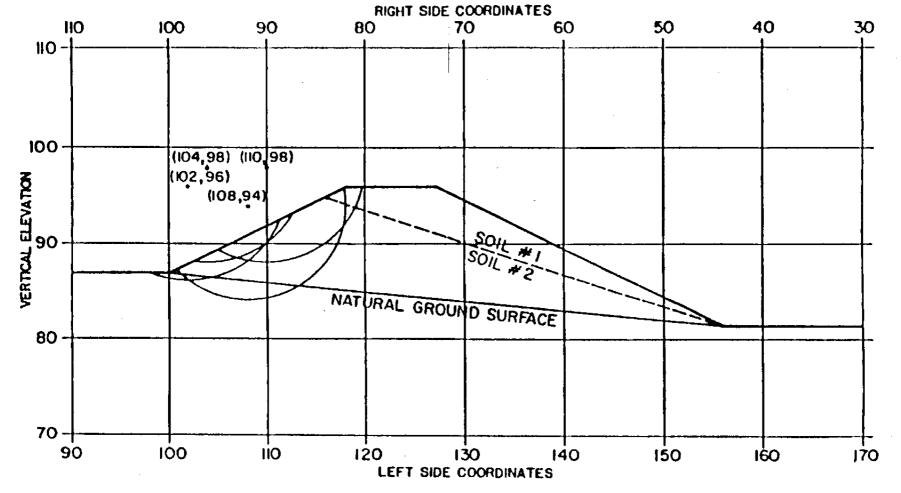
56145			To the state of th				
POINT	X-ORD	Y-ORD '					
1	0.00	81.58				•	
ノ ^{・2}	44.88	81.50					
3	73.00	96.00					
4	82.00	96.00					
5	84.00	95.00		•			
6	100.00	87.80					
7	200.00	87.00					
L	THE LEF		SOIL				
	1 1	2	2		•		
	2 2	5	2 2 2		•		
	3 5	6	2				
	4 6	7	2				
	3 5 4 6 5 2 6 3	3	1				
		4	1				
	7 4	5	1				
SOIL	UNIT	WEIGHT	COHESION	ø S	ATURATED		
1		18	788	29	но		
2		18	788	29	YES		
-	-						
CIRCLE	X-ORD	Y-ORD RA	DIUS FACTOR	OF SAFET	`Y		
	45.8		35.0 3				
SLICE	UEICHT	INCLINATION	COHESION	WIDTH	EFF WEIGHT	ø	×
1		-42.3	780	5.8	687.5		21.6
2	3665.2		700	5.0	1723.7	29	26.5
/ 3	5173.8	-22.7	700	5.0	2432.8	29	31.5
ر 4	6143.7	-14.1	700	5.0	2889.3	29	36.5
5	6653.8	-5.7	700	5.0	3129.2	29	41.5
6	8812.7	3.1	789	5.8	4315.1	29	46.9
7	10243.5	12.8	700	5.8	5329.1	29	52.7
8	10960.3	22.8	700	5.8	6007.3	29	58.5
9	10827.8	33.7	788	5.8	6286.1	29	64.3
1 Ó	9526.8	46.2	788	5.8	6015.0	29	70.1
11	4743.8		700	4.9	3409.3	29	75.4
12	198.1	72.5	788	1.0	190.1	29	78.3
4 €	1,70.1	12.5	. 00	1.0	170.1	- - ✓	
		• • • • • •	**	A.U. A.T.	•		
ITERAT		INITIAL	CHL	CULATED	•		
1		1.0000		3.1718	•		
2		3.1718		3.1953			
3	ŀ	3.1953		3.1962			

35

3.20 AT X=

BEAR CREEK CANYON MINE - SEDIMENTATION POND "A"

CROSS SECTION THROUGH EMBANKMENT

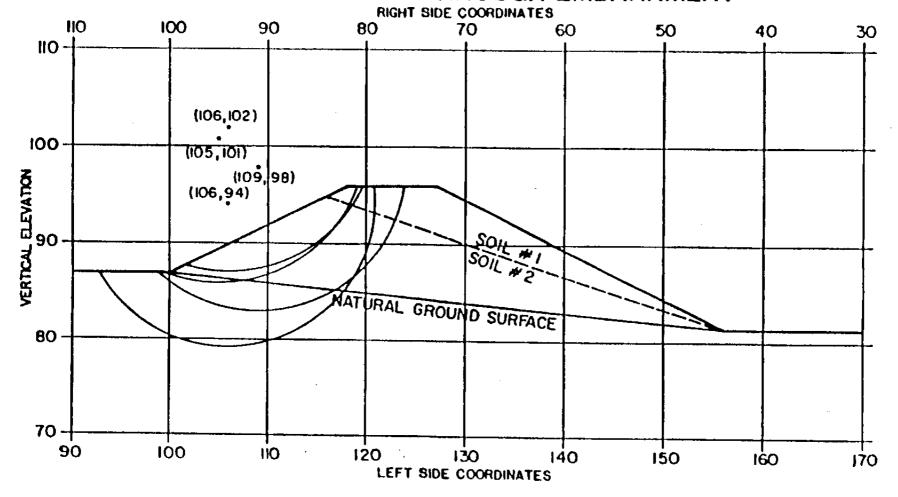


RADIUS = 10'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A" CROSS SECTION THROUGH EMBANKMENT

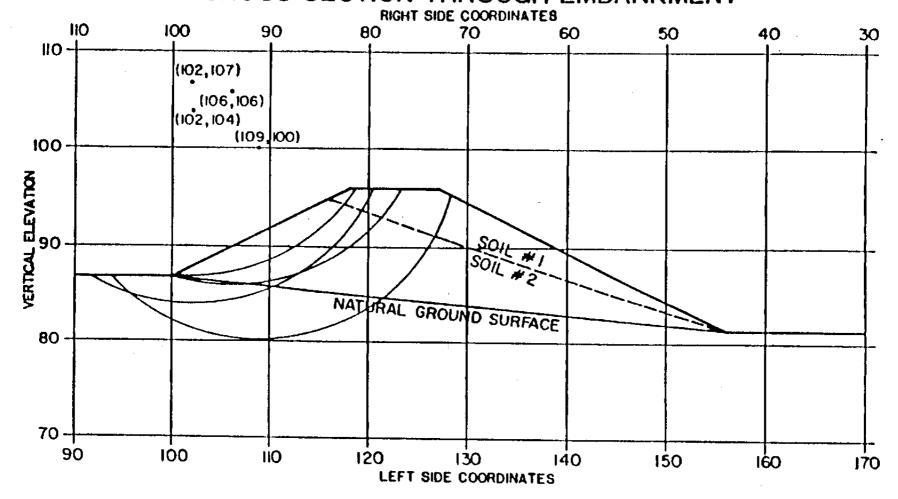


RADIUS = 15'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A" CROSS SECTION THROUGH EMBANKMENT



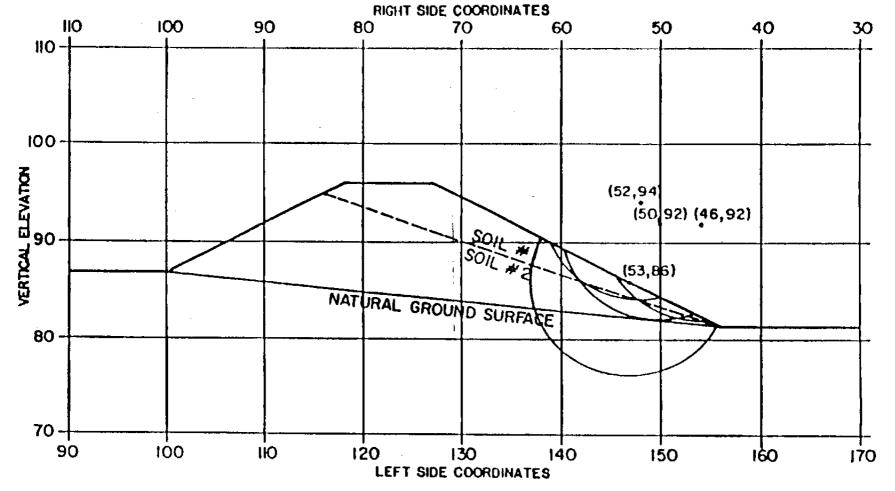
RADIUS = 20'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A"

CROSS SECTION THROUGH EMBANKMENT

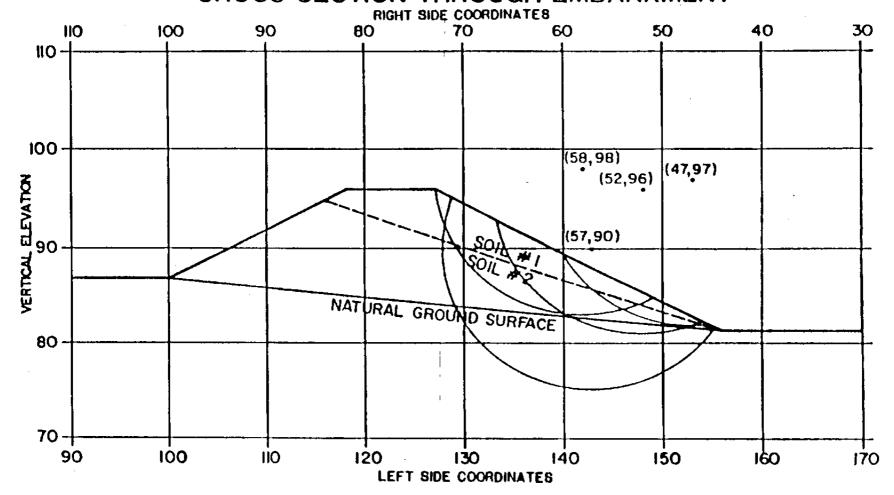


RADIUS = 10'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A" CROSS SECTION THROUGH EMBANKMENT



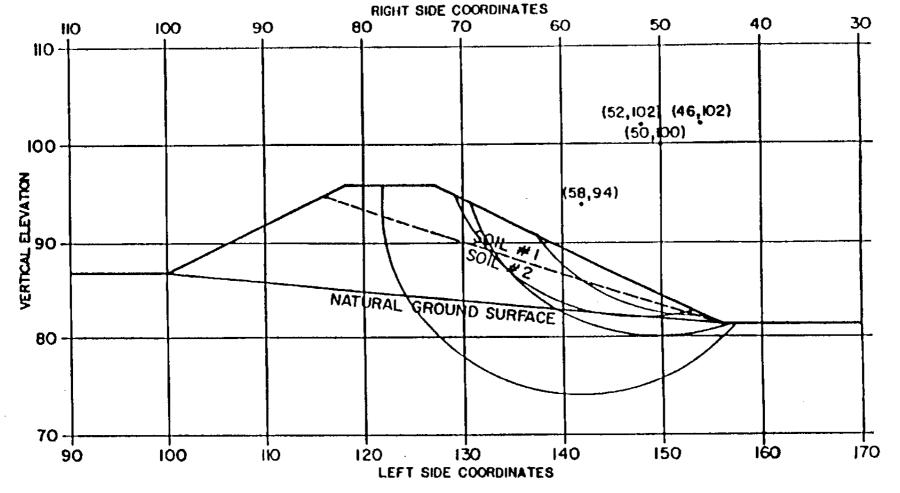
RADIUS - 15'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A"

CROSS SECTION THROUGH EMBANKMENT

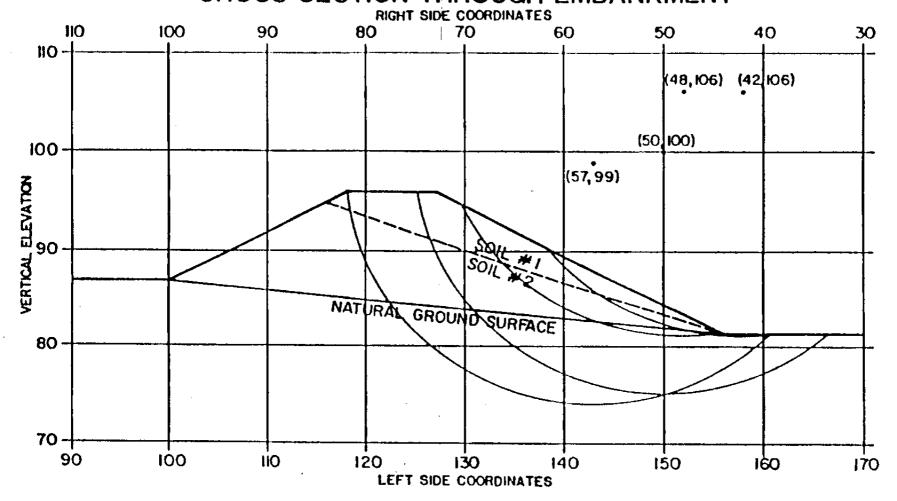


RADIUS = 20'

CO-OP MINING COMPANY



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A" CROSS SECTION THROUGH EMBANKMENT



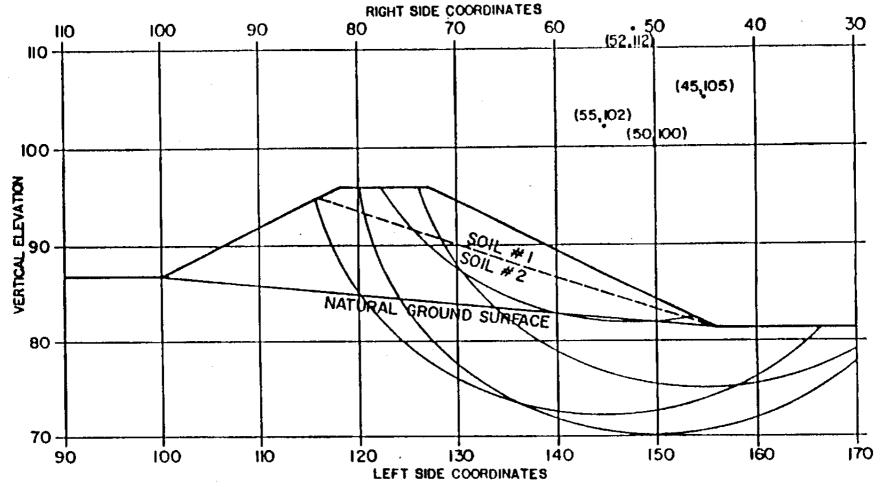
RADIUS = 25'

CO-OP MINING COMPANY

SCALE 1"- 10"



BEAR CREEK CANYON MINE - SEDIMENTATION POND "A" CROSS SECTION THROUGH EMBANKMENT



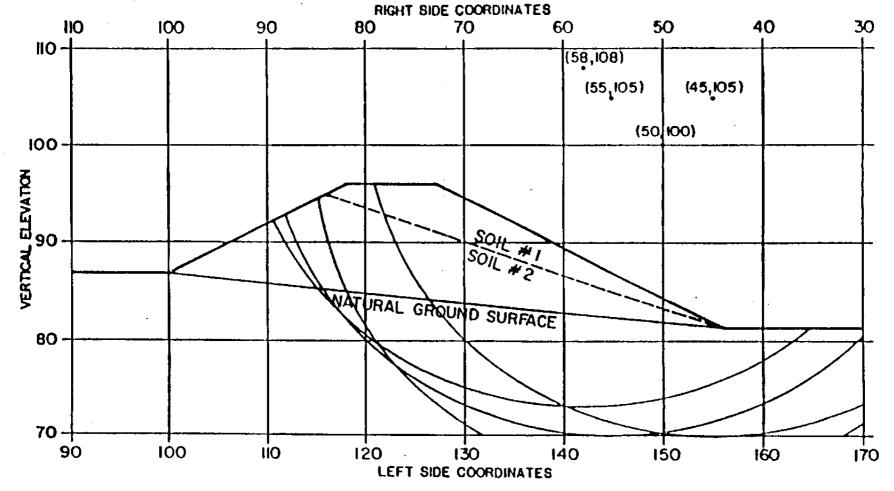
RADIUS = 30'

CO-OP MINING COMPANY



BEAR CREEK CANYON-MINE - SEDIMENTATION POND "A"

CROSS SECTION THROUGH EMBANKMENT



RADIUS = 35¹

CO-OP MINING COMPANY

SCALE 1"- 10"



"SLOPE"

A slope stability program utilizing the simplified or "modified Sishop" method.

The program was written by John P. Cross, P.E., Processing Hanager of STS. Consultants, Northbrook, Illinois. This program was printed in the October 1982 issue of "CIVIL ENGINEERING."

This version was copied from "CIVIL ENGINEERING" and edited for the Hewlett-Packard 9845 desk-top computer by Horrocks Engineers in March 1983. The format for the input and the output was changed from the original version, however, the program itself was not changed.

HORROCKS ENGINEERS ONE WEST MAIN STREET AMERICAN FORK, UTAH 83003 TELEPHONE (801)756-7628

```
42
      OPTION BASE 1
44
      OVERLAP
46
      PRINTER IS 16
48
      PRINT "SLOPE STABILITY ANALYSIS"
50
      DIM P(50,2),L(50,3),S2(5,4),A(50),F(50,7),Z(50,4),H$[80],Sbit$(0:1)[3]
52
      INTEGER Logo(2)
54
      Sbit$(1)=" NO"
56
      Sbits(0)="YES"
-8
      S9=10
      J6=8
 2
      OUTPUT 5: "R"
64
      ENTER 5: M, D, Times
66
      Dates=VRLs(H)&"/"&VRLs(D)&"/83"
68
      PRINTER IS 8
78
      PRINT =",LIN(4), TAB(80-LEN(Date$)); Date$,LIN(8);
72
      GOSUB Logo ! PRINT HORROCKS_ENGINEERS' LOGO
      PRINT LIN(5), TAB(28), "SLOPE STABILITY ANALYSIS", LIN(2), TAB(38), "for", LIN(2
74
);
```

```
*** INPUT OF PROGRAM VARIABLES ***
```

INPUT "ENTER THE DATA FILE HAME", Files

INPUT "ENTER THE PROJECT NUMBER", Pos

78

79

82

```
INPUT "ENTER THE USER'S INITIALS". User$
186
      LINK Files, 5000
≥196
      READ H$
191
      PRINT TAB(40-LEN(H$)/2);H$
195
      PRINT LIN(36), TAB(36), "DATA FILE: "&CHR$(34)&Files&CHR$(34), LIN(1)
266
      PRINT TAB(25), "PROJECT NUMBER: "&Pns, LIN(1)
201
      PRINT TAB(37), "by: "&User$
263
      PRINTER IS 16
210
      READ SO
      IF S0=0 THEN 270
211
```

3 READ S6 READ S7 <u>...</u>ê READ HO .98 READ E1 310 READ PI

311 PRINTER IS 16 315 PRINT "POINT X-ORD Y-ORD" 320 FOR I=1 TO PI

PRINT SPACES IT 331

```
3. .
           IMAGE 3X, 2CTD, 3D, 2X/
  350
           READ P(1,1),P(1,2)
  371
           PRINT USING 332; P(1,1), P(1,2)
  380
         HEXT 1
  466
         READ LI
  401
         PRINT LIN(1), "LINE
                                FROM
                                           TO
                                                 SOIL BENEATH
  402
         IMAGE 3X,2(4D,3X),2X,2D
  418
         FOR 1=1 TO L1
  421
           PRINT 1:
  448
           READ L(I,1),L(I,2),L(I,3)
  489
           PRINT USING 402; L(1,1), L(1,2), L(1,3)
  498
         HEXT I
         READ SI
  510
  5.1 1
         PRINT LIN(1), "SOIL
                              UNIT WEIGHT
                                             COHESION
                                                          "&CHR$(218)&"
         IMAGE 3X,40.00,2X,90,3X,30,3X,3A
                                                                           SATURATION-
  512
  528
        FOR I=1 TO S1 -
  531
           PRINT I:
  550
           READ $2(1,1),$2(1,2),$2(1,3),$2(1,4)
          PRINT USING 512;52(1,1),52(1,2),52(1,3),5611$($2(1,4))
  610
  620
        HEXT I
  *** CIRCLE DEFINITION
 648
        F9=0
 641
        PRINTER IS 16
 650
        PRINT FCIRCLE DEFINITION
        INPUT "ENTER THE X-ORD, Y-ORD, AND RADIUS OF THE FAIL SURFACE FORMAT X,Y,
 RT. X, Y, R
     CHECK TO SEE IF CIRCLE EXCEEDS TOP LINE END POINTS ++4
       U1=P1...
.730
 740
       FOR I=2 TO P1
 758
          IF (P(I,1)<P(I-1,1)) AND (U1=P1) THEN 778
 768
          GOTO 788
 770
          U1 = I - 1
 780
       HEXT I
798
       J1=R=R-(P(1,2)-y)-2
1866
       J2=R+R-(P(U1,2)-Y)-2
```

818

838

840

868 878

899

900

910

-⁄-∙0

40

950

968

978

988

990

'0

850 COTO 880

GOTO 4389

FOR I=1 TO L1

GOTO 978

S=9.99E18

COTO 1990

X1=P(L(I,1),1)

Y1=P(L(1,1),2)

X2=P(L(I,2),1)

Y2=P(L(1,2),2)

IF X2=X1 THEN 960

IF X2()X1 THEN 998

S=(Y2-Y1)/(X2-X1)

IF 314-8 THEN 838

IF J2<=8 THEN 850

820 . IF (J1>0) AND (P(1,1>>X-SQR(J1>) THEN 860

IF (J2>8) AND (P(U1,1)<X+SQR(J2)) THEN 868

DISP "CIRCLE EXCEEDS TOP LINE END POINTS";

DEFINE INTERSECTION OF CIRCLE WITH LINES ***

```
1010
           Cl=X1-Y1/S
  1010
         . 52×178/2+1
  1030
           C3=2+C1/S-2+X/S-2+Y
  1040
           C4=C1-2-2+X+C1+X-2+Y-2-R-2
  1056
           C5+C3^2-4+C2+C4
  1060
           IF CS(B THEN 1080
  1070
           GOTO 1898
  1086
           Z(1,1)=0
  1090
           IF C5(# THEN 1638
  1100
           Q1=(-G+SQR(C5))/(2+C2)
  1110
           92=(-G-SQR(C5))/(2+C2)
  1120
           03=01/$+C1
  1130
           Q4=Q2/S+C1
  1140
           GOTO 1248
  1150
           C5=R-2-(Y-Y1)-2
  1160
           IF C5(8 THEN 1180
 1170
          GOTO 1198
          Z(I,1)=0
 1188
 1190
          IF C5(8 THEN 1630
 1200
          93*X+SER(C5)
          @4=X-SBR(C5)
 1218
 1220
          Q1=Y1
 1230
          Q2=Y1
 1240
          J1-0
 1258
         . J2=8
          IF (ABS(S)(=9.99E9) AND (Q3>=X1) AND (Q3(=X2) THEN 1280
 1260
 1270
          GOTO 1298
 1289
          J1=1
          IF (ABS(S)(=9.99E9) AND (Q4)=X1) AND (Q4(=X2) THEN 1310
 1298
 1300
          GOTO 1328
 1310
          J2 = 1
          IF (S(-9.99E9) AND (Q1>=Y2) AND (Q1(=Y1) THEN 1348
  320
 4338
          GOTO 1350
 1340
          J1=1
         "IF ($(+9.99E9) AND (Q2>=Y2) AND (Q2(=Y1) THEN 1378
 1350
 1360
          GOTO 1386
 1370
          J2=1
 1380
          IF ($)9.99E9) AND (Q1>=Y1) AND (Q1<=Y2) THEN 1488
 1398
          GOTO 1418
 1400
          J1=1
 1410
         IF ($)9.99E9) AHD (Q2>=Y1) AHD (Q2(=Y2) THEN 1438
1428
         GDT0 1448 - :
1430
        · J2=1
1440
         Z(I,1)=J1+J2
1450
         IF J1=1 THEN 1478
1460
         GOTO 1488
1478
         Z(I,2)=03
1488
         IF J1=1 THEN 1500
1498
         GOTO 1518
1500
         Z(1,3)=G[
1518
         IF (JI=8) AND (J2=1) THEN 1538
1520
         GOTO 1548
1530
         2(1,2)=04
1540
         IF (J1=8) RND (J2=1) THEN 1560
         GOTO 1578
1550
1568
         Z(1,3)=Q2
1570
         IF (JI=1) AND (J2=1) THEN 1590
 580
         GOTO 1680
 198
         Z(I,4)=Q4
600
         IF (JI=1) AND (J2=1) THEN 1628
.610
         GOTO 1638
         Z(1,3)=92
1628
1630
      NEXT I
1648
      X4 = 0
1650
      X5=9.99E28
```

1650

11=1

```
414 0
         FOF 1=1 10 U1-1
   2280
           03=A(1+1)-A(1)
   2290
           Q4=1HT(Q3/Q2)+1
   2300
           C1=Q3/Q4
  2310.
           C2=A(1)
  2320
           FOR J=1 TO Q4
  2336
             IF JC04 THEN 2350
  2340
             COTO 2360
  2350
             11=11+1
  2368
             IF J(Q4 THEN 2380
  2370
             GOTO 2398
  2380
             8(I1)=C2+C1
  2390
             IF JC04 THEN 2410
  2488
             GOTO 2428
  2418
             C2=C2+C1
  2428
           HEXT J
  2438
        NEXT I
        FOR I=1 TO I1
  2440
  2450
          FOR J=1 TO I1-1
  2460
             IF A(J+1)>A(J) THEN 2500
  2478
             J1=8(J+1)
  2488
            A(J+1)=A(J)
  2490
            A(J)=J1
  2500
          HEXT J
  2519
       HEXT I
  *** DEFINE SOIL PARAMETERS FOR EACH SLICE ***
 2530
        F1=11-1
 2540
        FOR I=1 TO F1
 2550
          F(I,4)=A(I+1)-A(I)
 2569
          X6=F(I,4)
 2578
          F(I,7)=(A(I+1)+A(I))/2
 2580
       ·--X3=F(I,7) ····
 2598
          Y1=Y-50R(R-2-(A(I)-X)-2)
 2600
          Y2=Y-SQR(R^2-(A(I+1)-X)^2)
 2610
          A5=ATH(ABS(Y2-Y1)/F(I,4))
 2620
          IF Y2<Y1 THEH 2648
 2639
          G010 2658
 2640
          85=-85
. 2659
        ..F(1,2)=85
 2660
         IF A5=0 THEN 2680
 2678
         GOTO 2698
 2688
         F(I,2)=1.0E-5
 2698
         Y3=Y-SQR(R-2-(X3-X)-2)
 2700
         14-0
 2710
         FOR J=1 TO L1
2729
           L5=L(J,1)
2738
           L6=L(J,2)
2748
           IF (P(L5,2)(=Y3) AND (P(L6,2)(=Y3) THEN 2848
2750
           IF (P(L5,1)(X3) AND (P(L6,1)(X3) THEN 2848
           IF (P(L5,1)>X3) AND (P(L6,1)>X3) THEN 2848
2760
           Y6=P(L5,2)+(P(L5,2)-P(L6,2))/(P(L5,1)-P(L6,1))+(X3-P(L5,1))
2770
2780
           IF Y6<=Y3 THEH 2848
2790
           I4=I4+1
2800
           Z(14,1)=Y6
  310
           Z(I4,2)≈L(J,3)
/320
           W=0
:838
           E=8
2848
         HEXT J
2850
         IF 14=1 THEN 2970
2860
         FOR J=1 TO 14
2870
           FOR J1=1 TO 14-1
2886
```

IF Z(J1,1))=Z(J1+1,1) THEN DOWN

```
10 = 2 + 11 , 2 +
 2910
              2(31,1)*2(11*1,1)
 2910
              Z(J1,2)*Z(J1+1,2)
 こうらい
              Z(JI+1,1)=L5
  ن به ن
              Z(J1+1,2)=L6
295e
           HEXT JI
 2960
         HEXT J
2970
         14 = 14 + 1
2980
         Z(14,1)=Y3
2998
         FOR J1=1 TO 14-1
3000
           IF (1=1) AND (J1=1) AND (X3)=56) THEN 3020
3010
           GOTO 3838
3028
           [6=58-Y1
3030
           IF (I=F1) AND (J1=1) AND (X3)=S6) AND (X3(=S7) THEN 3858
3048
           GDTO 3860
3856
           J6=S8-Y2
3060
           H=H+(Z(J1,1)-Z(J1+1,1))+X6+$2(Z(J1,2),1)
3878
           IF (Z(J1,1)<S0) AND (X3)=S6) AND (X3(=S7) THEN 3090
3689
           GOTO 3198
3090
           W=H+(S0-Z(J1,1))+X6+W0
3100
           IF $2(Z(J1,2),4)>.95 THEN 3120
3116
           GOTO 3138
3120
           E4=S2(Z(J1,2),1)
3130
           IF $2(Z(J1,2),4)<.95 THEN 3150
3148
           COTO 3160
3150
           E4=S2(Z(J1,2),1)-W8
3160
           E=E+(Z(J1,1)-Z(J1+1,1))+X6+E4
3170
         HEXT JI
3180
         F(1,1)=W
3198
         F(1,5)=E
3200
         F(I,3)=$2(Z(I4-1,2),2)
 210
         F(1,6)=2+PI+(S2(Z(14-1,2),3)/360)
3220
       HEXT I
3221
       NORMAL
3238
       IF F9=0 THEN 3360
3248
       PRINT USING 3250; CHR$(210)
3250
       IMAGE "SLICE
                       WEIGHT
                                 INCLINATION
                                                COHESION
                                                            WIDTH
                                                                      EFF WEIGHT
                                                                                     * 8
         X.
3288
       D=368/(2*PI)
3290
       FOR I=1 TO F1
3300
         PRINT USING 3328; I, F(I, 1), F(I, 2)+0, F(I, 3), F(I, 4), F(I, 5), F(I, 6)+0, F(I, 7)
         IMAGE 30,100.0,70.0,120,90.0,110.0,70,70.0
3328
3340
      HEXT I
3350
      PRINT
3360
       D=0 .
3361
      PRINTER IS 0
3378
      FOR I=1 TO F1
3388
         D=D+F(I,1)+SIN(ABS(F(I,2)))+(F(I,2)/ABS(F(I,2)))
3390
         D=D+E1+F(I,1)+COS(ABS(F(I,2)))
3400
      HEXT I
3418
      IF 16>0 THEN 3430
3420
      GOTO 3448
3438
      I7=W0+I6+I6+(R-I6/3)/(2+R)
3448
      IF 16>0 THEN 3460
3456
      GOTO 3478
3460
      D=D-SGH(1)+17
3470
      IF (16>0) AND (F9=1) THEN 3490
 188
      GOTO 3518
 198
      PRINT USING 3500;17
      IMAGE ** DRIVING FORCE COUNTER BALANCE OF *, 100.20
 ,60
 510
      IF J6>0 THEH 3530
3528
      GDTO 3548
3530
      I7=W0+J6+J6+(R-J6/3)/(2+R)
      IF J6>0 THEN 3560
3540
3556
      GOTO 3578
```

3560

D=D+SCN(D)+17

```
2500 GOTO (610
2590 PRINT PSING 3600;17
3600 IMMGE TORIVING FORCE INCREASE OF ", 100.20
```

IMAGE 48,70.20,70.20

PRINT USING 4100; I, P(I, 1), P(I, 2)

FOR I=1 TO PI

REXT I

PETNT

4100 4110

4120

4139

4146

```
--- ITERATIVE SOLUTION FOR FACTOR OF SAFETY ***
 3620
       F0=1
 3630
       R4=6
 3648
       16=0
 3650
       FOR I=1 TO F1
         R1=F(1,3)+F(1,4)+F(1,5)+TAH(F(1,6))
 3660
 3678
         R2=1/COS(ABS(F(I,2)))
 3680
         R3=1+TAN(F(1,6))+TAN(F(1,2))/F0
 3690
         R4 = R4 + R1 + (R2/R3)
 3700
       NEXT I
 3718
       F2=R4/D
 3720
       16=16+1
 3738
       IF F9=1 THEN 3750
 3748
       GOTO 3820
       IF 16=1 THEN 3770
 3750
 3768
       COTO 3880
3778
       PRINT
       PRINT USING 3790
3780
       IMAGE "ITERATION", 11X, "INITIAL", 10X, "CALCULATED"
3790
       PRINT USING 3810; 16, F0, F2
3866
3818
       IMAGE 3X, 3D, 13X, 3D. 4D, 12X, 3D. 4D
3820
       IF 16>18 THEN 3848
3839
       GOTO 3850
3840
       PRINT "WILL NOT CLOSE"
3850
       IF 16>18 THEH 3970
 1868
       IF ABS(ABS(F0)-ABS(F2))(.005 THEN 3900
-3870
       F0=ABS(F2)
3880
       R4=0
3898
       COTO 3658
3900 !
3901
       IF NOT F9 THEN
3982
         PRINTER IS 16
3903
       ELSE
3984
         PRINTER IS 8
3985
       END IF
3918
      PRINT
      PRINT USING 3930; F2, X, Y, R
3920
      IMAGE "FACTOR OF SAFETY" ",50.20," AT X= ",40," Y= ",40, " R= ",40
3938
3940
       PRINT USING 3950; E1
      IMAGE "EARTHQUAKE" ", 20.20
3958
3951
       IF F9 THEN 4380
3960
      PRINT
3961
      A$= " "
      INPUT "DO YOU WISH A FORMAL PRINTOUT (Y/N)", AS
3970
      IF UPC$(R$[1,1])="N" THEN 4320
3990
3991
      PRINTER IS 0
4838
      IMAGE @"NATER UNIT WEIGHT=",3D.2D
4848
      PRINT USING 4030; HO
4841
      IF SO THEN
4050
        PRINT
4060
         IMAGE "SUBMERGENCE AT "3D.2D,"
                                          FROM
                                                 ".3D.1D."
                                                           TO
        PRINT USING 4060; 50, 56, $7
 978
  71
      END IF
986
      PRINT
      PRINT " POINT
-898
                       X-ORD
                                  Y-ORD"
```

```
4124
        F + 1111 1 1
                     LIME
                             LEFT
                                              _01k_"
  4160
        IMAGE 4.8 TO
  4170
        FOR I=1 TO LI
  4180
          PRINT'USING 4160; 1, L(1, 1), L(1, 2), L(1, 3)
  4190
        HEXT I
  4200
        PRINT
  4210
        PRINT "SOIL
                           UNIT WEIGHT
                                              COMESSION
                                                            "ECHR#(210)&"
 ED-
 4228
        IMAGE 30,150,170,90,7X,3A
 4238
        FOR I=1 TO S1
          PRINT USING 4220; I, $2(I, 1), $2(I, 2), $2(I, 3), $5(t$($2(I, 4))
 4248
 4250
        HEXT I
 4268
        PRINT
        PRINT *CIRCLE
 4278
                                           RADIUS FACTOR OF SAFETY"
                        X-ORD
                                  Y-ORD
        IMAGE 120.0,70.0,70.0,80.20
 4288
 4290
       PRINT USING 4288; X, Y, R, F2
 4300
       PRINT
 4318
       PRINT
 4311
       As=""
      INPUT "DO YOU WISH A DIACHOSTIC RUN (Y/N)", As
 4328
 4348
      IF UPC$(R$[1,1])="N" THEN 4370
 4360
       F9=1
 4378
       IF UPC$(A$[1,1])(>"H" THEH 720
 4371
       R$= " "
       INPUT "DO YOU WANT TO CONTINUE (Y/N)", 8$
 4388
 4488
      IF UPC#(A#[1,13)<>"N" THEH 630
 4481
      DISP * FINISHED *
 4418
      STOP
 4520 Logo: PLOTTER IS 13, "GRAPHICS"
 4530
      GRAPHICS
4540
       SCALE 8,559,8,454
4550
      LORG 2
 4560
      FOR I=0 TO 5
4578
         Logo(1)=-2175
4588
         Logo(2)=-4352
4598
         R=454-I
4608
         GLOAD Logo(+),0,R
4610
      HEXT I
4628
      FOR I=6 TO 14
4638
        Logo(1)=-2115
4648
        Logo(2)=-4352
4650
        R=454-I
4660
        _GLÖAD_Logo(+),0,R
4678
     HEXT I
4680 FOR I=15 TO 21
4698
        Logo(1)=-2175
4788
        Logo(2)=-4352
4718
        R=454-I
4720
        GLOAD Logo(#>,0,R
4738
      NEXT I
4740 CSIZE 15/4.54,9/15
4758 MOVE 27,458
4760
      LABEL *HORROCKS*
4778
     MOVE 27,437
4780
     CSIZE 15/4.54,8/15
4798
      LABEL "ENGINEERS"
      BUMP GRAPHICS 430,454
4800
4818
      CCLEAR
828
      EXIT GRAPHICS
330
      RETURN
```

SATURAT

Slope stability program

JOHN P. CROSS, P.E., M. ASCE Data Processing Manager, Project Economic STS Consultants 1 Northbrook, Briggs

FOR NATURAL OF man-made slopes, the index of stability with respect to a sudden failure is known as the safety factor of the slope. The safety factor may be defined as the ratio of the potential resisting forces to the drive forces tending to cause movement. A slope on the verge of failure would have a safety facfor of 1.0. The analysis of slope stability is, therefore, the analytical procedure of determining the most critical, i.e., the lowest, factor of safety of given or proposed slope

Manual methods of slope stability analysis were developed prior to the advent of the electronic computer. These approaches resulted in high analysis costs and conservative slope configurations. Repetitive calculations lended themselves to computerized methods and numerous programs exist that have been written for large computer systems to perform slope stability analysis according to a number of theoretical methods.

The simplified or modified Bishop method is reasonably accurate for most purposes where the slope under analysis can be assumed to fail along a circular failure surface. The factor of safety is defined as the ratio of the resisting moments to driving moments around the center of the failure arc. Initially, a cross-section of rthe slope is drawn detailing soil strata and piezometric surfaces. A center point is then chosen from which an are is taken through the cross-section. This are represents the faijure surface: under evaluation. This failure zone is broken down into a series of slices which can be individually evaluated for their weight and strength characteristics. An illustration of a slope cross-section being defined by a series of slices is shown in Figure 1.

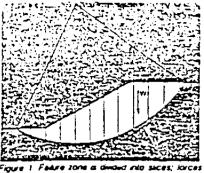
The forces acting on each slice are illustrated in Figure 1, where AX is the width of the slice. W is the weight of the slice. T is the force acting along the failure surface at the bottom of the slice. N is the effective force acting normaily to the base of the slice and O is the inclination of the failure surface or slice base. The factor of safety is defined as:

$$F = \frac{\sum (C \Delta X + N \tan \Phi) \sec \theta}{1 + \tan \Phi \tan \theta}$$

$$E = \frac{F}{\sum W \sin \theta}$$

Where C is the cohesion, Φ is the friction angle and the summation occurs over each slice of the failure zone. As the factor of safety, F, occurs on both sides of the equation. An interactive solution where F is initially estimated and then back substituted until the calculated F and estimated F close within a specified tolerance.

The equation can be modified to handle two additional conditions by adding additional factors to the term defining the driving force. These two conditions are standing pools, i.e., submergence of a portion of the slope, and earthquake loading. For submergence, the weight of water acting above the slice is added to the weight of the slice itself. The total driving force is increased or de-



creased by the weight of water above or below the exit of the failure surface from the slope. The second condition of earthquake loading can be handled by increasing the driving force calculated for each slice by EWcoso, where E is the earthquake loading factor. Similarly the resisting force is decreased by a decrease in the normal force due to the earthquake loading.

Following the calculation of the safety factor for this are, the center or radius of the are is modified to generate a new failure surface. The previousiv mentioned procedure is again followed with a new factor of safety being deter-

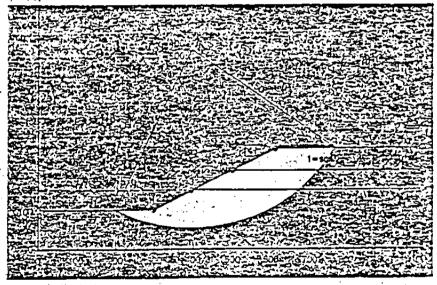


Figure 2, shows a hypical cross-section and the input parameters required to define the cross-section for the

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1140 60 10 1240
1150 C5=672-67-76777
1168 IF C5/0 IMEM 1189
1170 60 ID 1190
1180 FF12-11+0
1190 IF C3/0 IMEM 1620
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720 Fallet #4017FBOR X-ORD *1
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240 PRINT $4015 10 3-069, 11
250 lefut $40157
                                                                                                                                            240 PRINT PART 10 1-000."1

250 PRINT PART 10 1-000."1

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                                                                                                                                     1840 IF C540 THEM 1880 (4)
```

mined. This entire sequence is repeated until the failure surface for the minimum factor of safety is determined.

The program included in this article follows the same general procedure as previously defined. The program can be broken down into nine segments. Lines 100-620 are input routines for the entry of data defining the cross-section, lines 630-710 define the circle that will gen-

erate the failure surface, lines 720-860 perform a verification that the failure are falls fully within the cross-section and lines 880-1840 define the intersection points between the line segments and the failure are. The slice array is set up between line 1850 and 2220, with slice boundaries defined in lines 2230-2510. Lines 2520-3600 include the definition of the soil parameters for

A STORY

each slice and the actual iterative solution for the factor of safety occurs between lines 3610 and 3950. The remainder of the program is the formal output of the results.

The program includes a diagnostic print-out where all the slice parameters can be displayed for any given failure surface. As currently configured the program can handle models including

```
1239 (E. 19 or tokin China
Craw fallet PROLITE TOLING TOWN
3250 INNUE TRUETE WEIGHT ENCLEMATION THE
3250 INNUE TRUETE STORY 32701
3270 INNUE TOWNSTON WINTER CYF MELGIN
                                                                      2078
            2078 REJESTANCES

2080 ACRE J

2100 MERT J

2110 MERT I

2120 FOR [#1 30 FT+1

2130 [F ACRIMATION IMER 2150
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                         3294 0-JA0/(20PI)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                             3200 FOR 1-1 TO F1
3300 PRINT 000: USING 3320:1:F(1-1)-F(1-2)-0.F(1-3)-F(1-4)-F(1-4)-F(1-3)
3310 PRINT 000: USING 3310:F(1-4)-0.F(1-7)
3310 PRINT (30.070.10-070.10-120-070.10-070.10-3
3310 PRINT 000:
3310 P
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       3290 FOR 1-1 TO FT
                 2140 60 10 2140
               2130 M:-01+1
2140 F ALTICATION 1MEM 2180
2170 60 TO 2170
| 100 AUDITACIO | 1334 Print quel | 1344 Print q
               2100 6(UL)=4(1)
2170 MEST 1
1700 FILES THEM 1779

1700 GO TO 1800 TO 1800 TO 1770 TO 1800 
       2430 IF 12CT1 1800 1440
2430 50-T1 2430 50 T1 2430 50 T
       2700 [Fre/L3:1300 AND PLA-1703 | THE 1700 F | 2700 F | 27
          2800 2114,53-745
2810 2(14,2)-14,6
          2020 | 100 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 | 204 |
       2890,13-201747

2700,14-2017214

2710,2017411-2014477

2710,20174711-2014477

2710,20174711-13-15-2
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       4250 MET I 4240 PRINT 8491
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     4300 PRINT #401
A310 PRINT #401
A320 PRINT #401
A320 PRINT #401
A320 REVIT #401
A330 IF 450*** THEN 4340
A330 RE OF 4320
A340 F9-1
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     4300 IL WEST. JICH 930
       3200 F(1,3)=62(Z(24-1,2)-27
3210 F(1,4)=62(Z(24-1,2)-27
3210 F(1,4)=7#P[0#2(Z([4-1,2),3)/340]
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 THEN ASA
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ne france de la france de la come despeta el Republic de despetations de la come despeta el france de la MATER WALT WE BORE 42.4 ESTIMUME 8.85 E-048 0 7-010 100 POINT--040 190 -OFR 108 E-049 466 T-000 (150 Y-048 200-1 7(-DRE:1909 HUNDER OF CTHES 4 €03 L.7 € **ACTION FACA** TO THE ME

ATOMINATION OPTIMENDO

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up to 20 points. 20 lines, and S wai types. The minimum number of slaces is set at 10, but can be changed by modifying the value of S9 at line 150 and changing the dimensioning of arrays A, F and Z.

A cross-section is drawn of the slope showing all soil strata and piezonietric surfaces. Each intersect point between lines on the cross-section is numbered, with the constraint that all points along the top line must be numbered consecutively from left to right initiating with point 1. Points beneath the top line may be numbered in any order. These points are then defined with X and Y coordinates (the entire cross-section must fall in the first quadrant).

Lines are then specified by assigning line numbers for each line occurring between two end points. These lines are defined by a left point, a right point and the number of the soil type occurring beneath the line. Vertical lines are not allowed and should be modeled by offsetting the X-coordinate of one end point by a small amount, i.e., .01. Piezometric surfaces within the cross-section are treated as any other soil strata interface with saturated soil beneath and unsaturated soil above the line segment. Soil types are defined by specifying a unit weight, cohesion, phi angle and an indication of whether the soil is saturated

supplied includes the unit weigh of water, the earthquake loading factor and information defining any standing pool of water. This pool is specified by inputting the Y-coordinate of the pool

Additional information that must be

elevation and the left and right X-coordinates defining the extent of the pool. The only remaining input parameters are the center coordinates and radius of

the failure are. The input can be in any consistent set of units.

Figure 2 shows a typical cross-section and the input parameters required to define the cross-section for the program. The sample execution in the box shows the typical input sequence and output formats. This result is not necessarily the minimum factor of safety for this slope, but the factor of safety for the specified failure surface. Additional runs should be made using different centers and radii until the minimum factor of safety is located.

The program presented here is the nucleus from which system specific enhancements should be made. These enhancements are not included in this version for the purpose of minimizing the size of the program presented. Particularly valuable enhancements include the ability to edit interactively, save input on a disc or tape and perform a search for the minimum factor of sufety without manually inputting ench circle definition. Additional enhancements could include plotting expabilities and more detailed diagnostic features.

more detailed diagnostic features.

A computerized evaluation of slope stability should never take place apart from a complete evaluation of the geophysical conditions involved. Likewise, the results should always be subjected to evaluation and interpretation based on current engineering practice and experience.

Cross received his M.S. degree in civil engineering from Rose-Hulman Institute of Technology. Terre House, Indiana in 1972. Currently data processing manager/project engineer, Cross has been with STS (formerly Soil Testing Services) for 10 years. He holds a Master of Dhinity from Trinity Evengelical Divinity School and is working on an MBA at the Keller Graduate School of Management.

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